

## **8.8 Roadway Overtopping**

### **8.8.1 General**

Roadway overtopping will begin when the headwater rises to the elevation of the roadway. The overtopping will usually occur at the low point of a sag vertical curve on the roadway. The flow will be similar to flow over a broad crested weir. Flow coefficients for flow overtopping roadway embankments are found in HDS No. 1, Hydraulics of Bridge Waterways, as well as in the documentation of HY-7, the Bridge Waterways Analysis Model. Curves from the latter are included in Figure 8-9. The roadway overtopping flow rate can be determined using the following equations.

$$Q_r = C_d L HW_r^{1.5} \quad (8.9)$$

Where:  $Q_r$  = overtopping flow rate,  $m^3/s$  ( $ft^3/s$ )

$C_d$  = overtopping discharge coefficient (weir coefficient) =  $k_t C_r$

$k_t$  = submergence coefficient

$C_r$  = discharge coefficient

$L$  = length of the roadway crest, m (ft)

$HW_r$  = the upstream depth, measured above the roadway crest, m (ft)

### **8.8.2 Length**

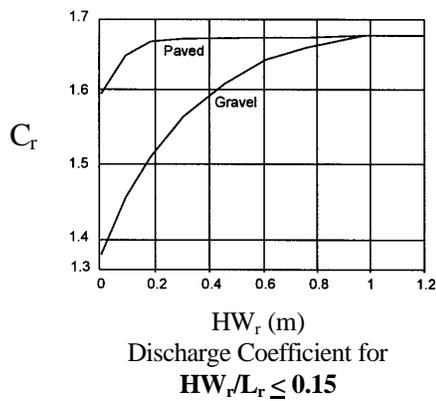
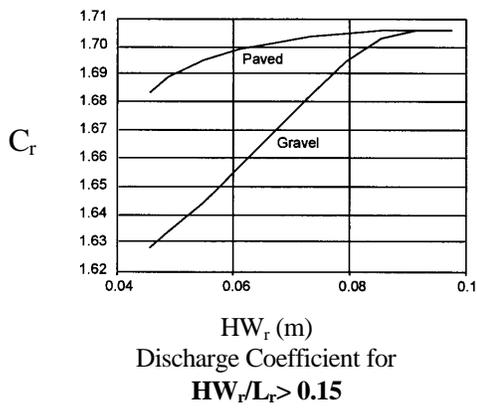
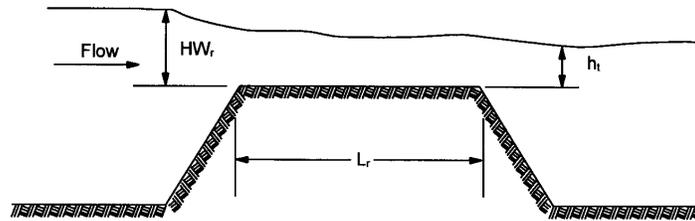
The determination of length versus HW is difficult to define when the length of the crest is defined by a roadway sag vertical curve.

- It is recommended to subdivide the length into a series of segments. The flow over each segment is calculated for a given headwater. The flows for each segment are added together to determine the total flow.
- As an approximation, the length can be represented by a single horizontal line (one segment). The length of the weir is the horizontal length of this segment. The depth is the average depth (area/length) of the upstream pool above the roadway.

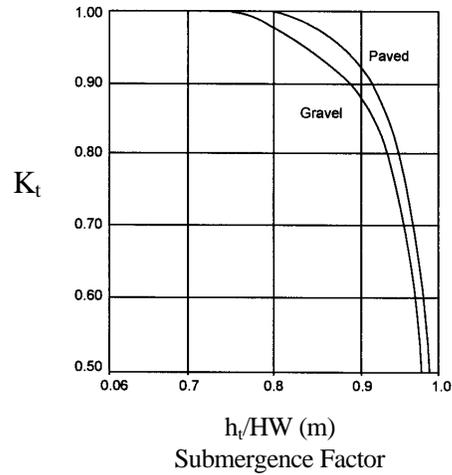
### **8.8.3 Total Flow**

Total weir flow is calculated for a given upstream water surface elevation using equation 8.9.

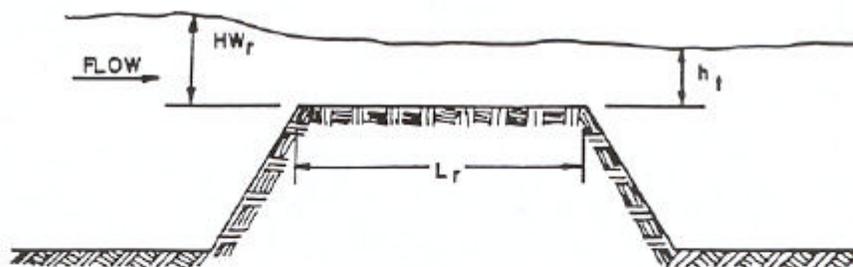
- Roadway overflow plus culvert flow must equal total design flow.
- A trial and error process is necessary to determine the flow passing through the culvert and the amount flowing across the roadway.
- Performance curves for the culvert and the road overflow may be summed to yield the overall performance characteristics at the crossing.



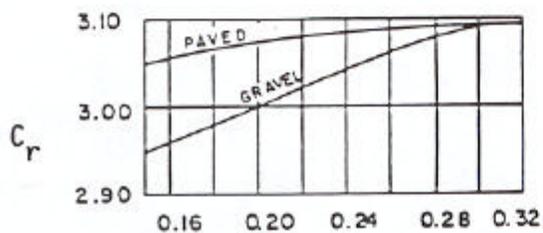
$C_d = k_t C_r$   
 $C_r$  = Coefficient of Free Discharge  
 $k_t$  = Adjustment Factor for Submerged Weir Flow  
 (TW is Higher Than Roadway Elevation)  
 $Q_r = C_d L HW_r^{1.5}$



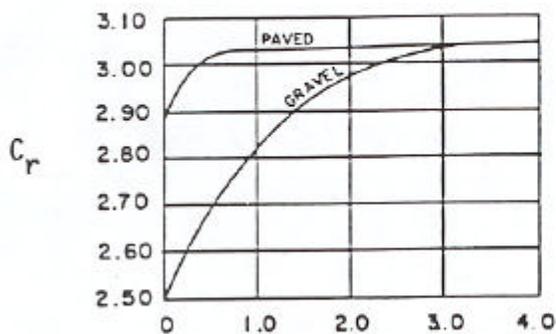
**Figure 8-9 Curves for Roadway Overtopping (Metric units)**



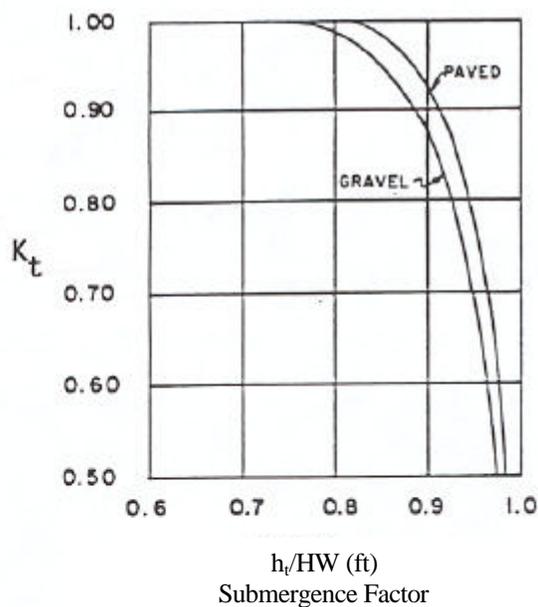
$C_d = k_t C_r$   
 $C_r$  = Coefficient of free discharge  
 $K_t$  = Adjustment factor for submerged flow  
 $Q_r = C_d L H W_r^{1.5}$



$HW_r$  (ft)  
 Discharge Coefficient for  
 $HW_r/L_r > 0.15$



$HW_r$  (ft)  
 Discharge Coefficient for  
 $HW_r/L_r \leq 0.15$



**Figure 8-9.1 Curves for Roadway Overtopping (English units)**