

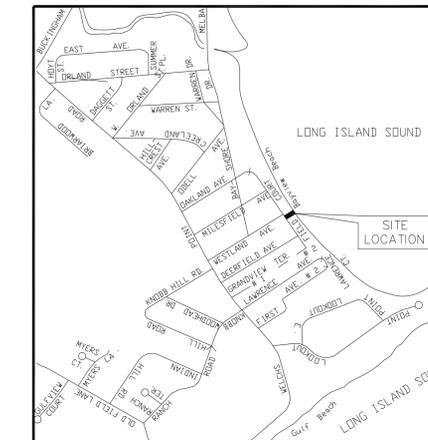
STRUCTURAL ENGINEER:



SURVEYOR & STORM WATER MANAGEMENT:



ENVIRONMENTAL ENGINEER:



LOCATION MAP
NOT TO SCALE

SITE PLAN NOTES

THIS SITE PLAN IS BASED ON A SURVEY THAT HAS BEEN PREPARED BY FREEMAN COMPANIES, LLC. IN ACCORDANCE WITH THE REGULATIONS OF CONNECTICUT STATE AGENCIES, SECTIONS 30-20-1 THROUGH 30-20-20 AND THE "STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT" ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INC. ON SEPTEMBER 26, 1996.

THE TYPE OF SURVEY IS A BOUNDARY AND TOPOGRAPHIC SURVEY. IT IS DEPENDENT IN NATURE AND BASED UPON MAP REFERENCE NUMBER ONE.

THE SURVEY ACCURACY CONFORMS TO HORIZONTAL CLASS "A-2", TOPOGRAPHIC ACCURACY CLASS T-2.

HORIZONTAL CONTROL AND MAP BEARINGS ARE BASED ON ASSUMED DATUM. VERTICAL CONTROL AND ELEVATIONS ARE BASED ON NAVD83 DATUM.

THE SUBJECT PROPERTY IS CURRENTLY OWNED BY SAUL G. ENGLANDER, SEE MILFORD LAND RECORDS VOLUME 3274 AT PAGE 469, AND IS LOCATED IN AN R5 ZONE.

THE EXISTING LOT IS NON CONFORMING TO THE CURRENT ZONING REGULATIONS FOR LOT AREA, WIDTH AND FRONTAGE. ITS USE IS DECLARED TO BE NONCONFORMING BUT NOT IN VIOLATION SINCE THE LOT EXISTED OF RECORD PRIOR TO MARCH 15, 1997. SEE SECTION 8-13g OF THE CONNECTICUT GENERAL STATUTES LISTED HERE FOR REFERENCE.

THE PROPERTY DEPICTED HEREON IS LOCATED IN FLOOD ZONE AE (BASE FLOOD ELEVATIONS DETERMINED) (ELEVATION 11') AND FLOOD ZONE VE (COASTAL FLOOD ZONE WITH VELOCITY HAZARD (WAVE ACTION); BASE FLOOD ELEVATIONS DETERMINED) (ELEVATION 13') BASED ON A VISUAL INSPECTION OF "FIRM FLOOD INSURANCE RATE MAP NEW HAVEN COUNTY, CONNECTICUT PANEL 533 OF 635 MAP NUMBER 09009C0533J MAP REVISED JULY 6, 2013 BY: FEDERAL EMERGENCY MANAGEMENT AGENCY."

"SEC. 8-13a. NONCONFORMING BUILDINGS AND LAND USES. (A) WHEN A BUILDING IS SO SITUATED ON A LOT THAT IT VIOLATES A ZONING REGULATION OF A MUNICIPALITY WHICH PRESCRIBES THE LOCATION OF SUCH A BUILDING IN RELATION TO THE BOUNDARIES OF THE LOT OR WHEN A BUILDING IS SITUATED ON A LOT THAT VIOLATES A ZONING REGULATION OF A MUNICIPALITY WHICH PRESCRIBES THE MINIMUM AREA OF THE LOT, AND WHEN SUCH BUILDING HAS BEEN SO SITUATED FOR THREE YEARS WITHOUT THE INSTITUTION OF AN ACTION TO ENFORCE SUCH REGULATION, SUCH BUILDING SHALL BE DEEMED A NONCONFORMING BUILDING IN RELATION TO SUCH BOUNDARIES OR TO THE AREA OF SUCH LOT, AS THE CASE MAY BE."

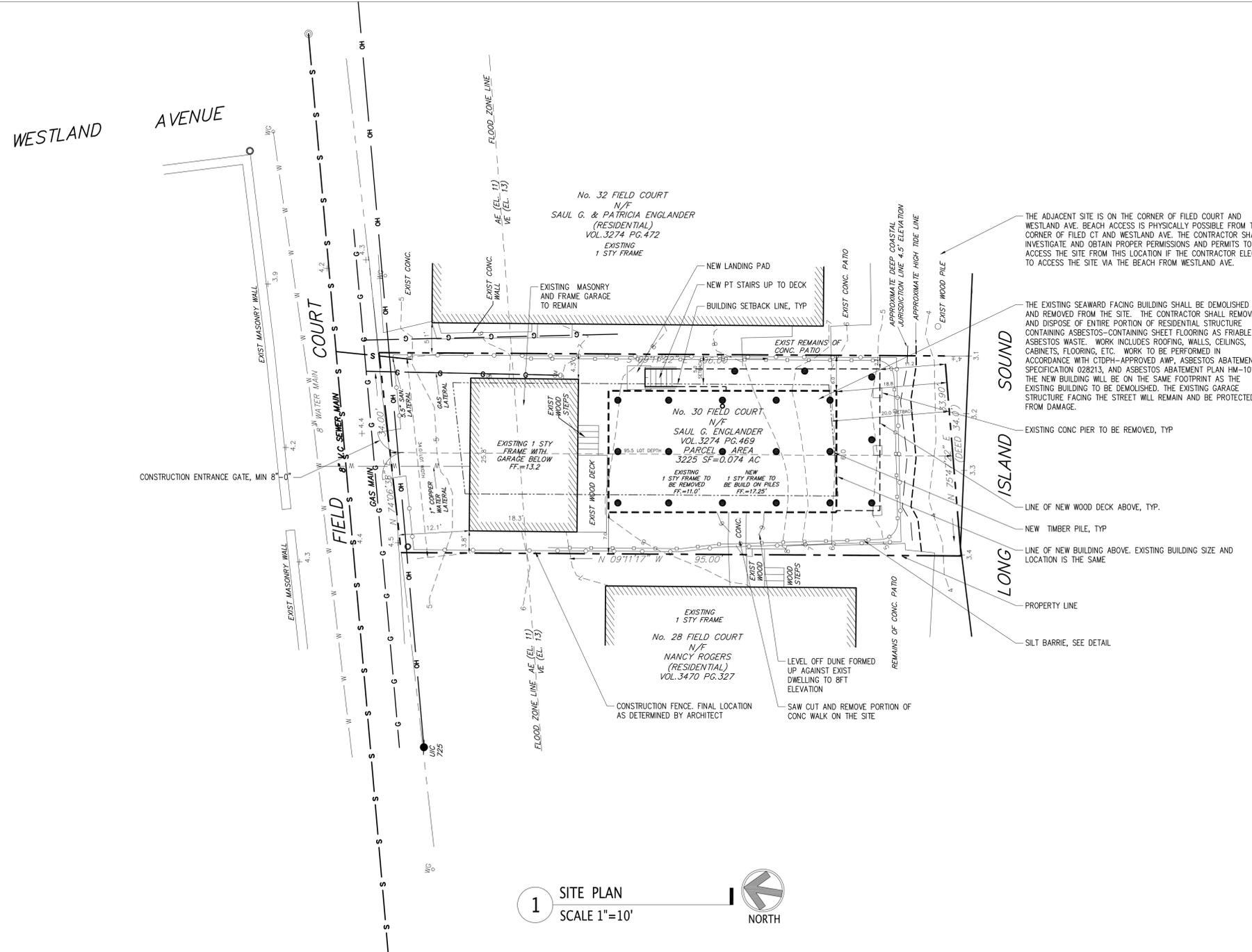
THE UNDERGROUND FEATURES DEPICTED HEREON ARE THE RESULT OF COMPILED OF EXISTING MAPPING AND LOCATION OF UTILITY PAINT. ACTUAL LOCATION OF UNDERGROUND UTILITIES IS TO BE CONSIDERED TO BE APPROXIMATE AT BEST. OTHER UTILITIES MAY EXIST WHICH FREEMAN COMPANIES ARE UNAWARE OF. VERIFY INFORMATION IN THE FIELD, BEFORE ANY DIGGING OR SITE EXCAVATION CALL "CALL BEFORE YOU DIG" 1-800-922-4455.

MAP REFERENCES

- "SURVEY OF PROPERTY PREPARED FOR SAUL G. ENGLANDER & PATRICIA A. ENGLANDER OF PARCEL DESIGNATED AS No. 30 FIELD COURT SITUATED IN THE CITY OF MILFORD, CONNECTICUT SCALE 1"=10' DATED JUNE 15, 2013".
- "MAP OF LOTS AT BAY VIEW, OWED BY GEORGE E. HASKINS AND H.C.C. MILES, MILFORD, CONNECTICUT DATED JANUARY 23, 1922".

LEGEND

---	PROPERTY LINE
---	CHAIN LINK FENCE
W	WATER LINE
S	SANITARY LINE
G	GAS LINE
M.P.	METAL POST
○	UTILITY POLE
◆	PROPERTY CORNER IRON PIN
□	CATCH BASIN
•	SANITARY MANHOLE
MB	MAILBOX
○	DECIDUOUS TREE
VOL.	VOLUME
P.C.	PAGE

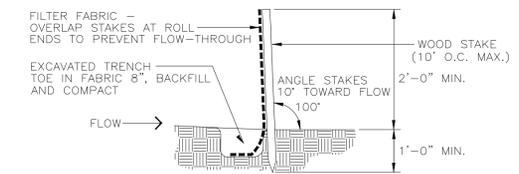


1 SITE PLAN
SCALE 1"=10'
NORTH

SITE PLAN INFORMATION TAKEN FROM A SURVEY PREPARED BY FREEMAN COMPANIES, LLC
MAP, BLOCK AND LOT: 28-574-5

R-5 ZONE	ZONING DATA		
	REQUIRED	EXISTING	PROPOSED
LOT AREA	5,000 SF	3,225 SF EXISTING NONCONFORMANCE	3,225 SF EXISTING NONCONFORMANCE
LOT WIDTH	50 Ft	34 Ft EXISTING NONCONFORMANCE	34 Ft EXISTING NONCONFORMANCE
LOT DEPTH	70 Ft	95.5 Ft	95.5 Ft
MAX No. OF STORIES PER BUILDING	3	2	2
MAX HEIGHT OF A BUILDING	35 Ft	23 Ft	24 Ft
FRONT YARD	12.1 Ft	12.1 Ft	12.1 Ft
SIDE YARD	ONE 10 Ft, ONE 5 Ft	3.8 Ft WEST, 4.3 Ft EAST EXISTING NONCONFORMANCE	3.8 Ft WEST, 4.3 Ft EAST EXISTING NONCONFORMANCE
REAR YARD	20 Ft	18.8 Ft EXISTING NONCONFORMANCE	18.8 Ft EXISTING NONCONFORMANCE
MAX LOT COVERAGE AS % OF LOT AREA	65%	45%	51%
BUILDING AREA AS % OF LOT AREA	45%	39%	39%

YARDS ARE TAKEN TO BUILDING WALLS.



- INSPECT BARRIER AFTER EACH STORM EVENT AND DAILY DURING PROLONGED RAINFALL.
- REMOVE SEDIMENT WHEN IT REACHES APPROXIMATELY ONE-HALF THE BARRIER HEIGHT.

1 SILT BARRIER
NTS

SITE PLAN ZONING DATA AND NOTES

PROJECT NO.: 1524-36 SCALE AS NOTED

DRAWING NO.:

G-002

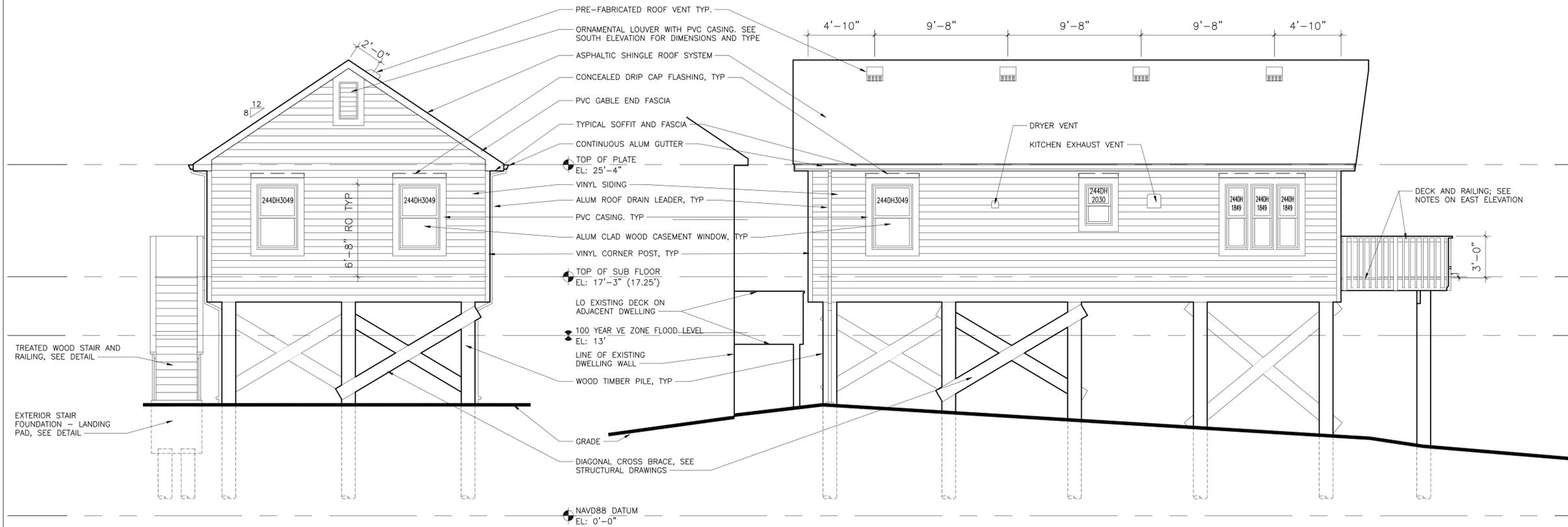
STRUCTURAL ENGINEER:



SURVEYOR & STORM WATER MANAGEMENT:

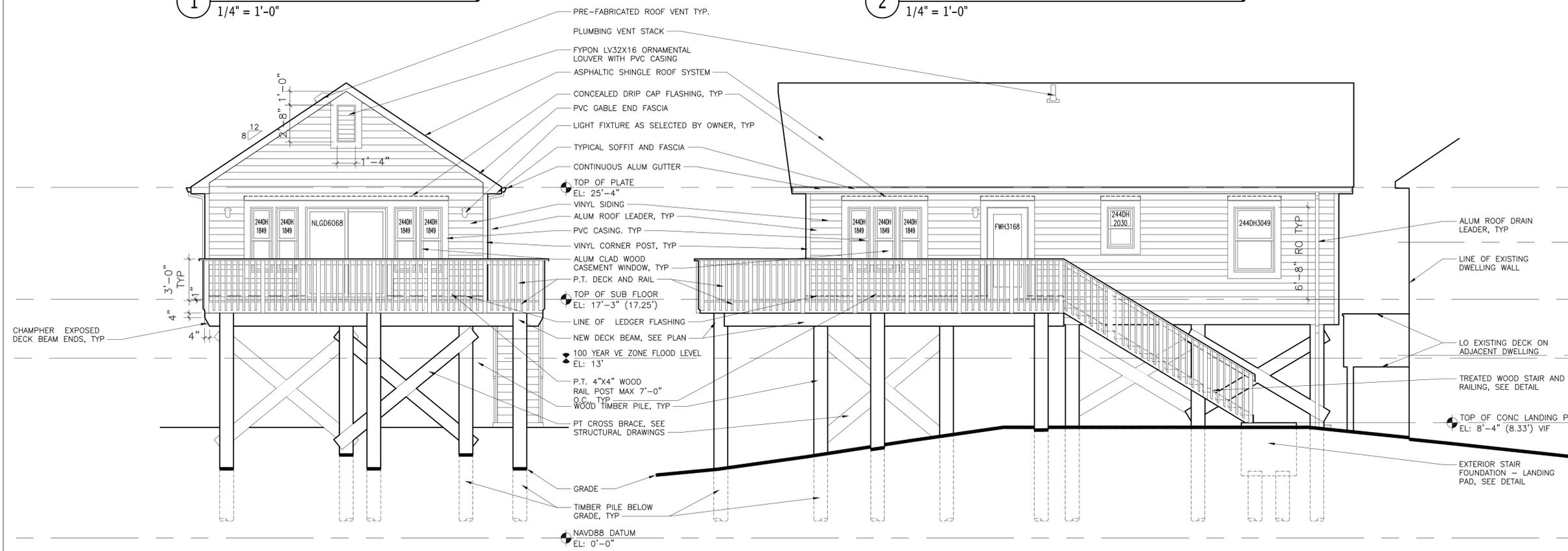


ENVIRONMENTAL ENGINEER:



1 NORTH ELEVATION
 1/4" = 1'-0"

2 WEST ELEVATION
 1/4" = 1'-0"



4 SOUTH ELEVATION
 1/4" = 1'-0"

3 EAST ELEVATION
 1/4" = 1'-0"

NOTE: ALL WINDOWS AND EXTERIOR DOORS SHALL BE ANDERSEN 200 SERIES AS NOTED BY PRODUCT NUMBER ON ELEVATIONS OR EQUAL APPROVED BY ARCHITECT

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 Department Of Housing
 505 Hudson Street
 Hartford, Connecticut 06106

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 FOR
 SAUL & PATRICIA ENGLANDER
 30 FIELD COURT
 MILFORD, CONNECTICUT 06460

ELEVATIONS

PROJECT NO.: 1524-36 SCALE AS NOTED

DRAWING NO.:

A-201

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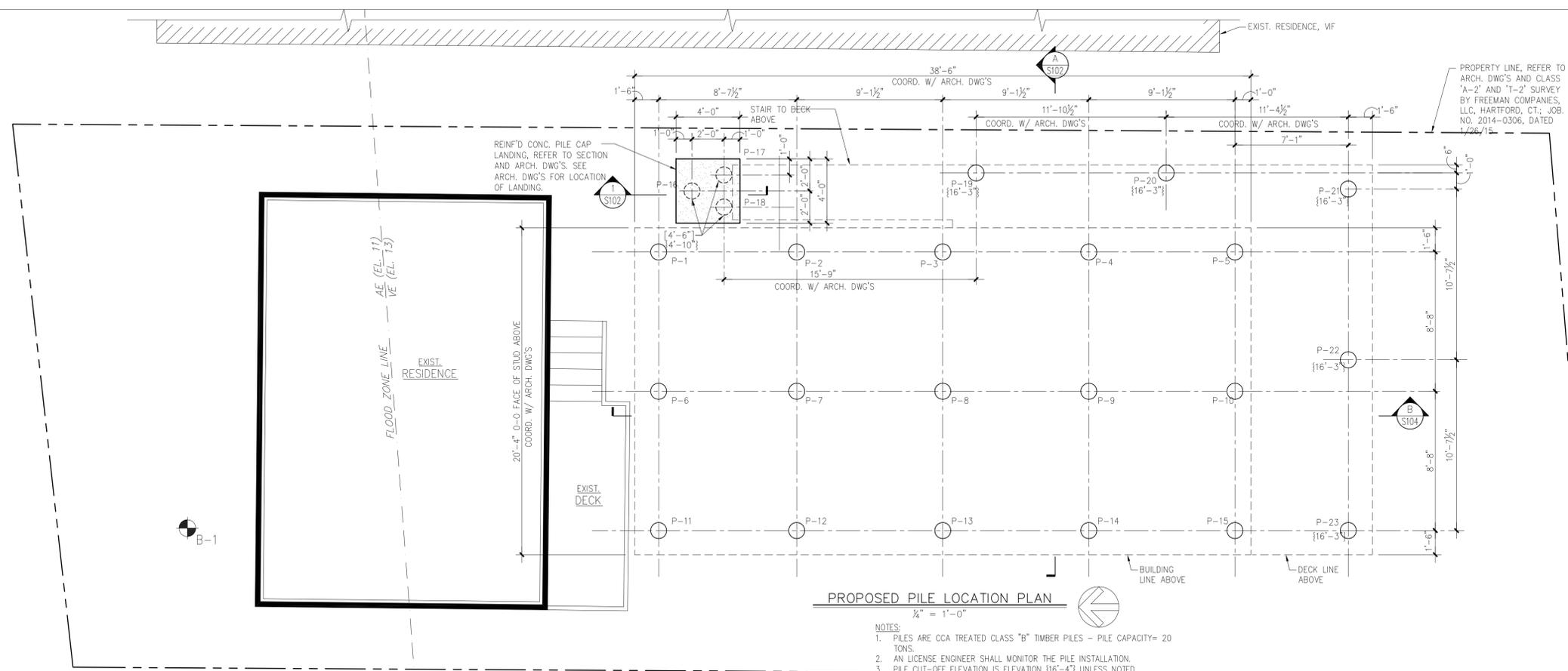
STRUCTURAL ENGINEER:



SURVEYOR & STORM WATER MANAGEMENT:



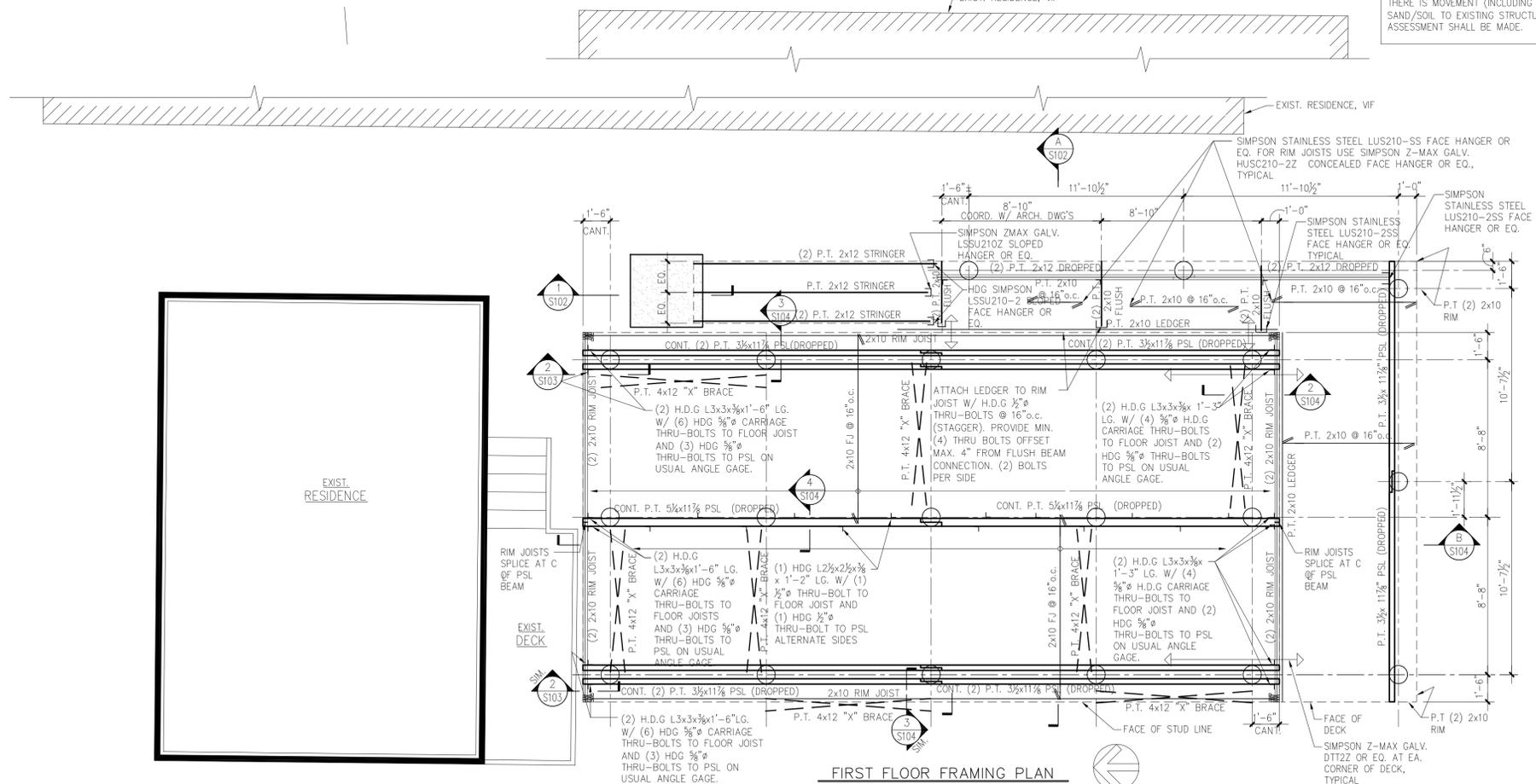
ENVIRONMENTAL ENGINEER:



PROPOSED PILE LOCATION PLAN
1/2" = 1'-0"

- NOTES:
- PILES ARE CCA TREATED CLASS "B" TIMBER PILES - PILE CAPACITY= 20 TONS.
 - AN LICENSE ENGINEER SHALL MONITOR THE PILE INSTALLATION. PILE CUT-OFF ELEVATION IS ELEVATION {16'-4"} UNLESS NOTED OTHERWISE ON PLAN, COORD. W/ ARCH. DWG'S.
 - BOTTOM OF PILE CAP ELEVATION NOTED THUS {±XX'-XX"} ON PLAN. PILE CUT OFF ELEVATION NOTED THUS {±XX'-XX"} ON PLAN.
 - "B-#": DENOTES BORING LOCATION REFER TO SHEET S-105.
 - COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS

NOTE:
IT IS RECOMMEND THAT PILE DRIVING CONTRACTOR TO MONITOR SOIL VIBRATION TO ADJACENT BUILDINGS WITH SEISMOGRAPHIC EQUIPMENT. CONTRACTOR SHALL STOP WORK AND NOTIFY ARCHITECT/ENGINEER OF RECORD IMMEDIATELY IF THERE IS MOVEMENT (INCLUDING VIBRATION) OF SAND/SOIL TO EXISTING STRUCTURES AND AN ASSESSMENT SHALL BE MADE.



FIRST FLOOR FRAMING PLAN
1/2" = 1'-0"

TYPICAL PSL SPLICE DETAIL
1/2" = 1'-0"

- NOTES:
- FLOOR FRAMING: 2x10 FLOOR JOISTS @ 16" o.c. ON P.T. PSL BEAMS PER PLAN. PROVIDE TOP AND BOTTOM BRACING AT 6'-0" o.c.
 - SUB FLOOR: 3/4" T&G APA RATED STURD-I-FLOOR, GLUED AND NAILED TO JOISTS WITH 8d NAILS @ 6" o.c.

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MILFORD, CONNECTICUT 06460

PILE LOCATION, PROPOSED FIRST FLOOR FRAMING PLAN

PROJECT NO.: 1524-36 SCALE AS NOTED

DRAWING NO. S-101

N:\10 PROJECTS\Housing-Residential\1524-36 - CT DDH-CBRC-BR - 30 Field Ct., Milford (#5073)\1524-36_DRAWINGS\1524-36_15_0728_Structural Drawings\15-110_S1.dwg

STRUCTURAL ENGINEER:



SURVEYOR & STORM WATER MANAGEMENT:



ENVIRONMENTAL ENGINEER:



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MILFORD, CONNECTICUT 06460

ROOF FRAMING PLAN,
BUILDING SECTION

PROJECT NO.: 1524-36 SCALE AS NOTED

DRAWING NO.:

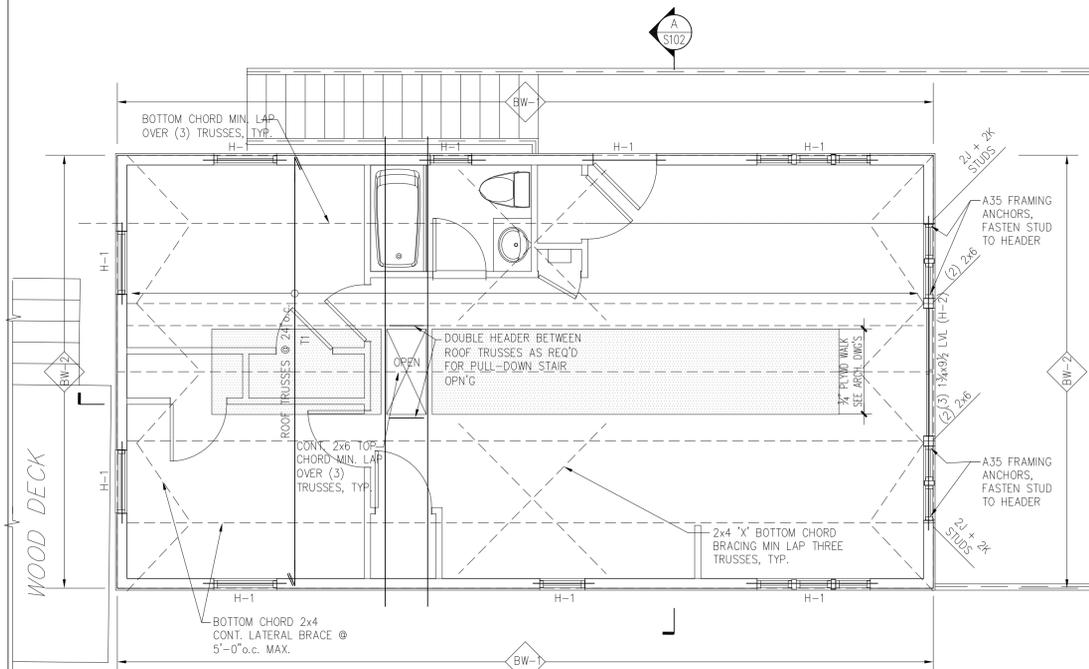
S-102

HEADER SCHEDULE			
MARK	HEADER	JAMB	
		JACK'S	KING'S
H-1	(2) 2x10	(2) 2x6	(2) 2x6
H-2	(3) 1 1/2" x 9 1/4" LVL	SEE PLAN	SEE PLAN

NOTE:
1. FOR EACH EXTERIOR HEADER CONNECTION REFER TO TYPICAL HEADER ANCHORAGE DETAIL ON SHEET S103

BEARING WALL SCHEDULE			
MARK		LOCATION	REMARK
BW-1	2x6 @ 16" o.c.	EXTERIOR BEARING	
BW-2	2x6 @ 12" o.c.	GABLE END	FULL HEIGHT STUDS TO UNDERSIDE OF ROOF FRAMING

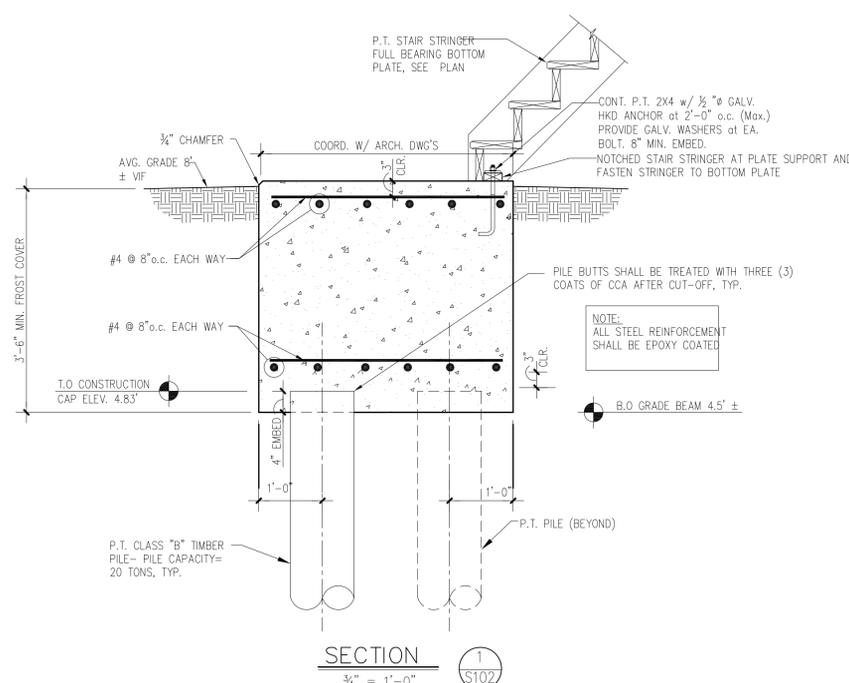
NOTE:
1. REFER TO ARCHITECTURAL DRAWINGS FOR ALL NON-STRUCTURAL WALLS.



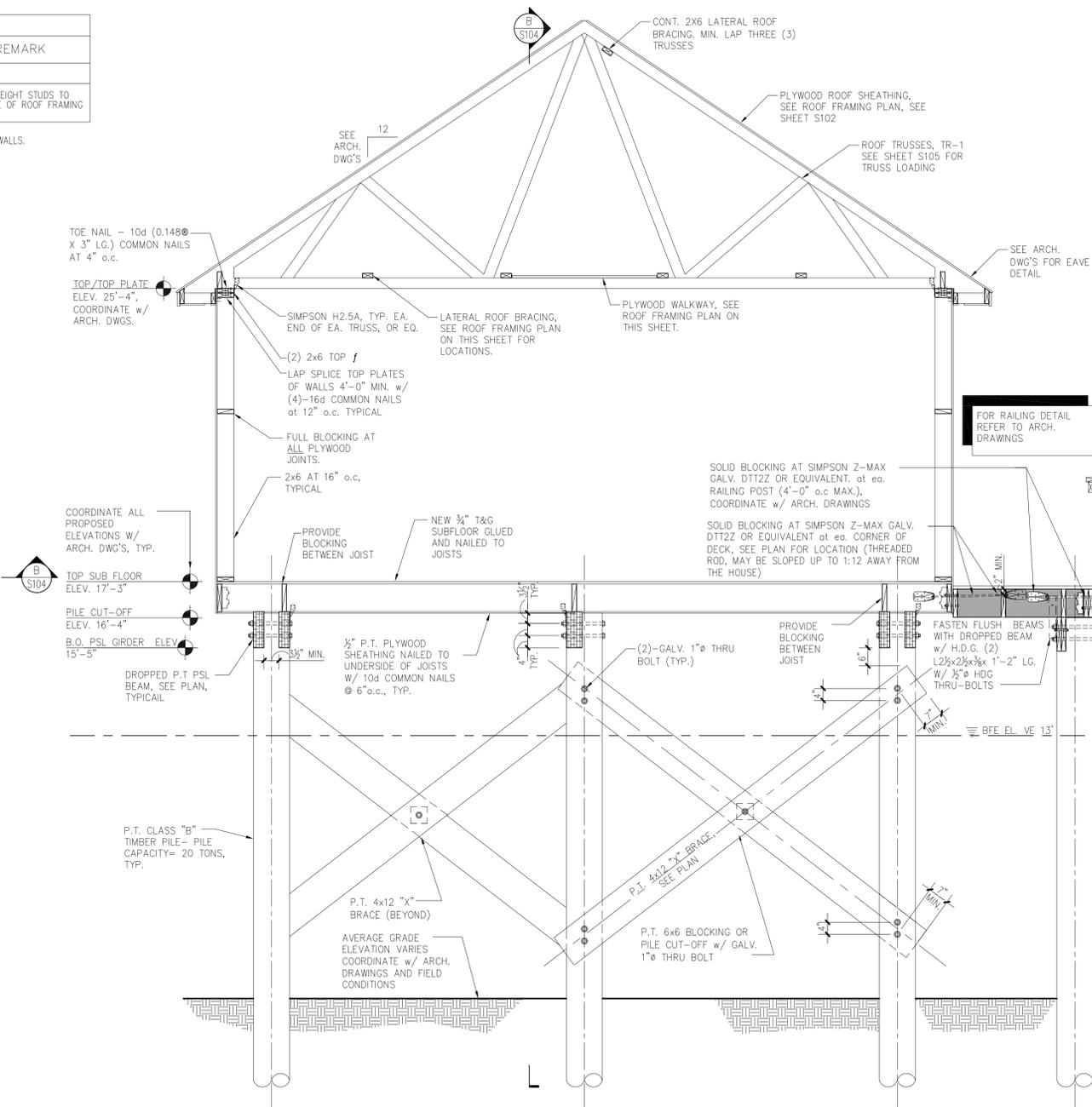
ROOF FRAMING PLAN
1/4" = 1'-0"

NOTES:

- ROOF FRAMING: PRE-FABRICATED WOOD TRUSSES @ 24" o.c. FASTENED TO DOUBLE TOP PLATE WITH SIMPSON H2.5AZ HURRICANE ANCHORS OR EQ., TYP. EA. END OF EA. TRUSS.
- ROOF SHEATHING: 3/8" APA RATED PLYWOOD SHEATHING FASTENED TO ROOF TRUSSES w/ 10d COMMON NAILS @ 6" o.c. AT PANEL EDGES, 10d COMMON NAILS @ 12" o.c. IN PANEL FIELD. PROVIDE PANEL EDGE CLIPS (20 GAGE H.D.G H-CLIPS) AT PANEL EDGES.
- PROVIDE TRUSS BRIDGING AND LATERAL BRACING PER TRUSS DETAILS DWG S-105.



SECTION
1/4" = 1'-0"



SECTION
1/2" = 1'-0"

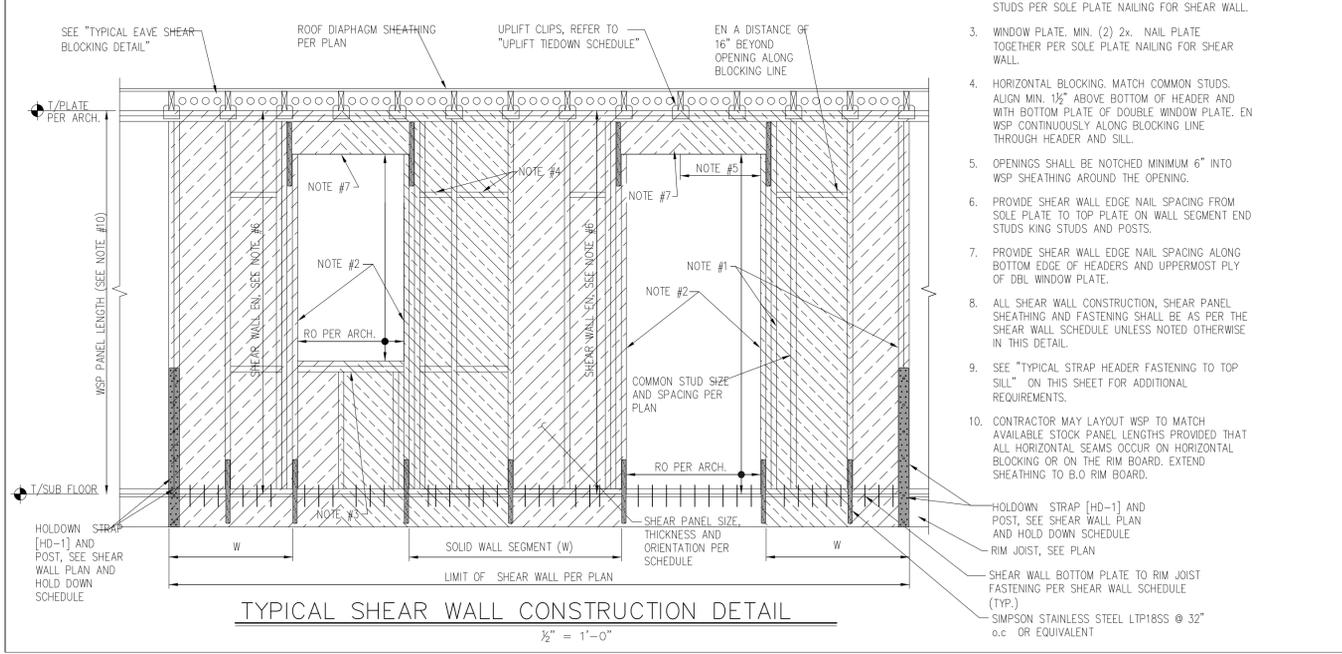
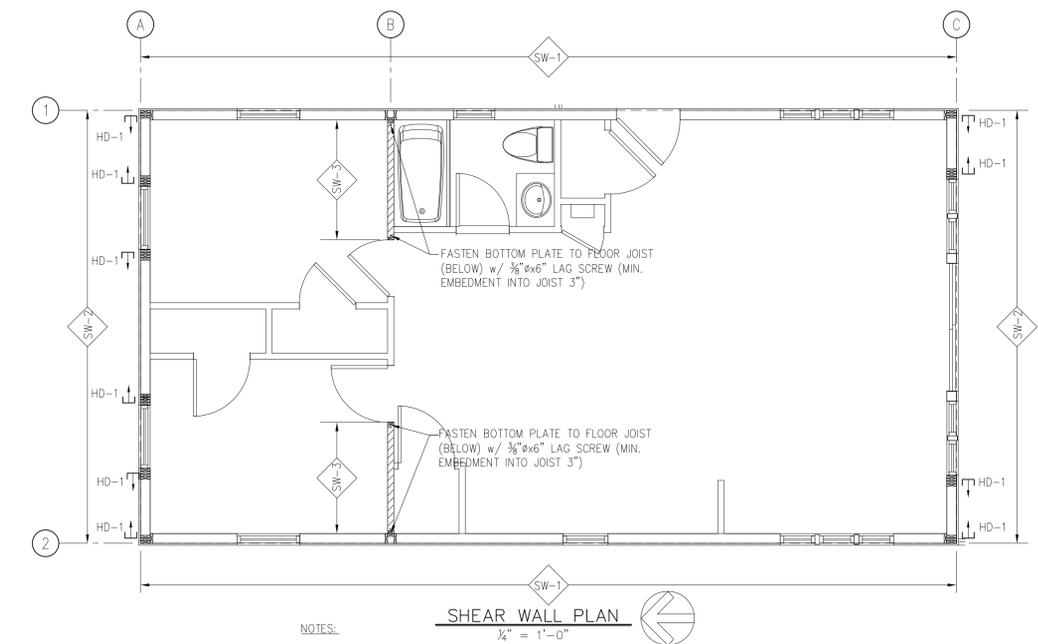
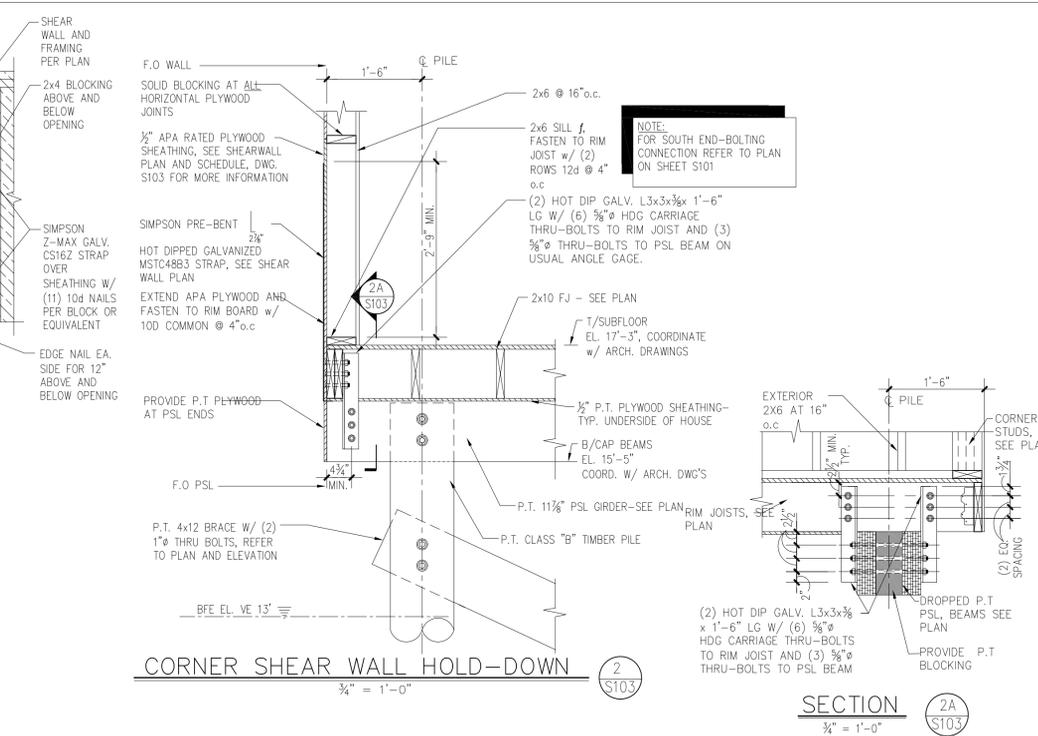
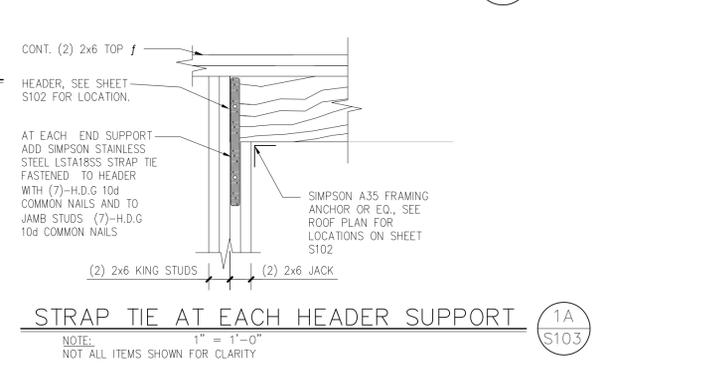
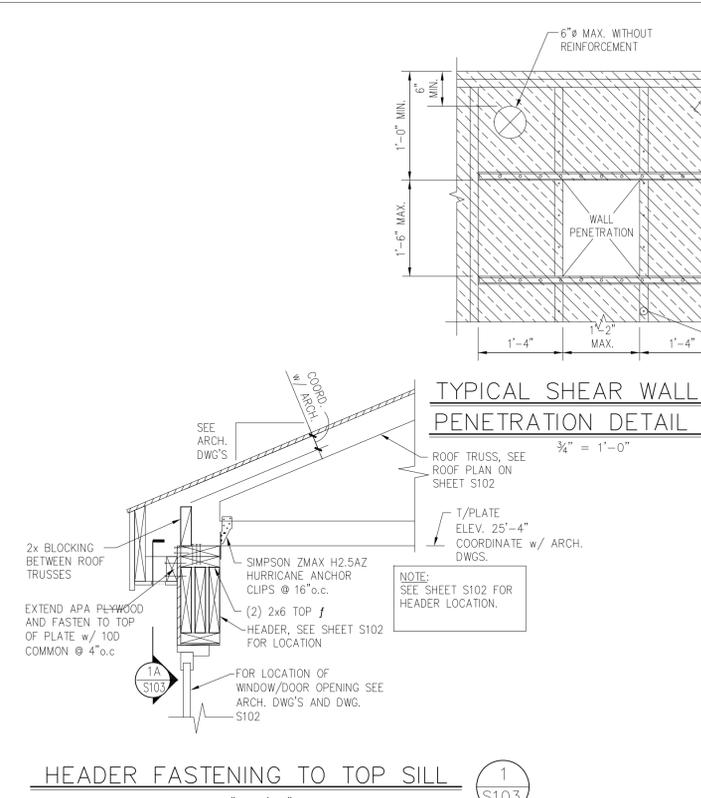
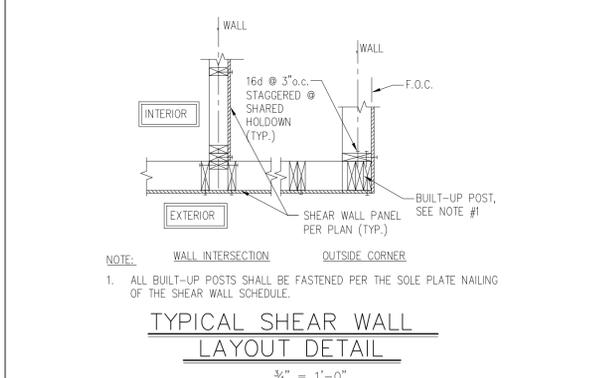
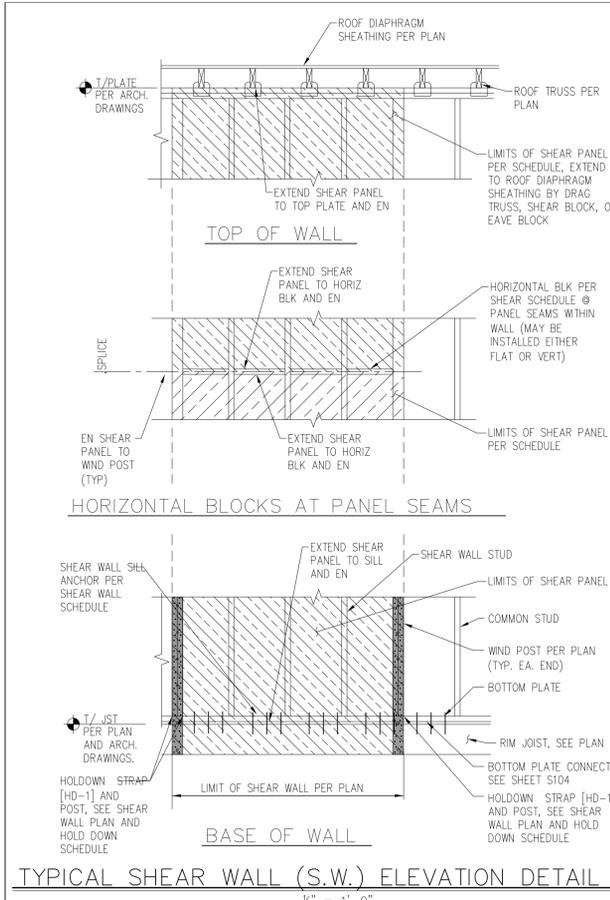
STRUCTURAL ENGINEER:



SURVEYOR & STORM WATER MANAGEMENT:



ENVIRONMENTAL ENGINEER:



NOTES:

- SHEAR WALLS TO BE SHEATHED ON OUTSIDE FACE OF WALL.
- PROVIDE HORIZONTAL BLOCKING AT ALL HORIZONTAL SHEATHING JOINTS.
- SHEAR WALLS LOCATED AT PLUMBING WALLS SHALL BE SHEATHED AND FASTENED PRIOR TO INSTALLATION OF ANY PLUMBING FIXTURES THAT WOULD REDUCE SHEATHING LIMITS OR BLOCK SHEATHING INSTALLATION.
- ALL SHEAR WALLS SHALL EXTEND UP TO THE UNDERSIDE OF ROOF DIAPHRAGM SHEATHING.
- FLOOR JOIST SHALL BE PLACED ALIGN INTERIOR SHEAR WALL.

SHEAR WALL SCHEDULE						
MARK/FACES	SHEATHING TYPE	SHEATHING THICKNESS (MIN.)	FASTENER TYPE [H.D.G.]	PANEL PERIMETER NAILING	PANEL INTERIOR NAILING (MAX.)	BOTTOM PLATE CONNECTION TO RIM JOIST NAILING
SW-1	ONE SIDE	APA WOOD	1/2 INCH	10d COMMON	6" o.c.	12" 4" o.c.
SW-2	ONE SIDE	APA WOOD	1/2 INCH	10d COMMON	4" o.c.	12" (2) ROWS AT 4" o.c.
SW-3	BOTH SIDES	GYPSON	1/2 INCH	#6 DRYWALL SCREWS	6" o.c.	12" 4" o.c.

HOLDOWN SCHEDULE						
MARK	HOLDOWN	FASTENERS			POST	SILL ANCHORAGE
		FACE	BOTTOM	STUDS		
HD-1	MSTC48B3	12- 10d H.D.G.	4- 10d H.D.G.	38- 10d H.D.G.	(2) 2x6	(2) 2x6

NOTES:

- HOLDOWNS SHALL BE HOT DIPPED GALVANIZED AND MANUFACTURED BY SIMPSON STRONG-TIE OR EQUIVALENT
- MIN. 21" END LAP FASTEN TO STUDS
- BUILT-UP MEMBER WITH SIMPSON SDS25300 SCREWS, SPACED 6" o.c. STAGGERED ALONG ENTIRE LENGTH OF MEMBER. MAINTAIN 1" MIN. EDGE SPACING, OR EQUIVALENT. FOR (3) STUDS FASTEN EACH FACE OF STUDS AS NOTED ABOVE.

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SAUL & PATRICIA ENGLANDER
30 FIELD COURT
MILFORD, CONNECTICUT 06460

SHEAR WALL PLAN, TYPICAL SHEAR WALL CONSTRUCTION, SECTIONS

PROJECT NO.: 1524-36 SCALE AS NOTED

DRAWING NO.:

N:\10 PROJECTS\Housing-Residentia\1524-36 drawings\1524-36_15_0728_Structural Drawings\15-110_S1.dwg

STRUCTURAL ENGINEER:



SURVEYOR & STORM WATER MANAGEMENT:



ENVIRONMENTAL ENGINEER:



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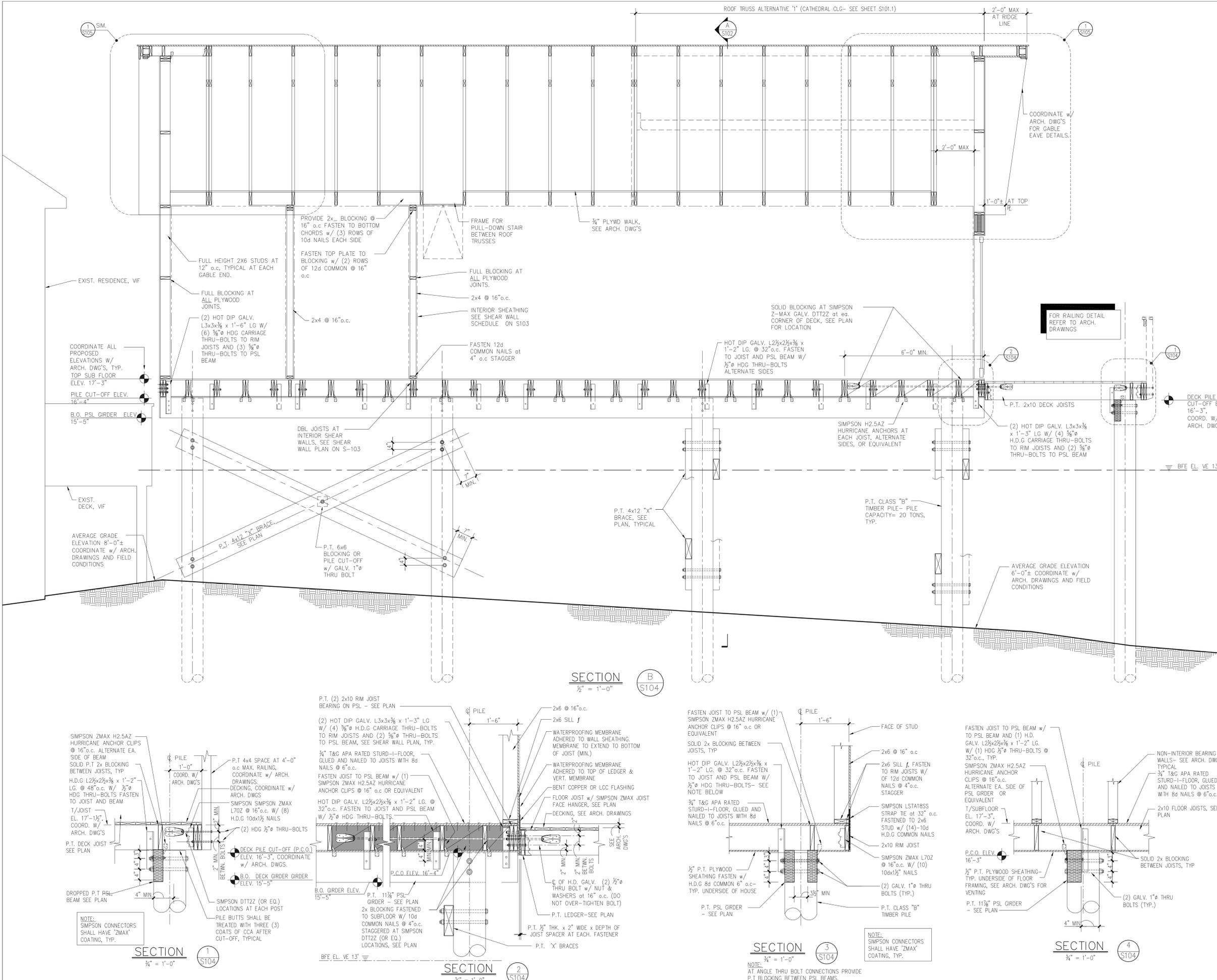
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STRUCTURAL
SECTIONS

PROJECT NO.: 1524-36 SCALE AS NOTED

DRAWING NO.:

S-104



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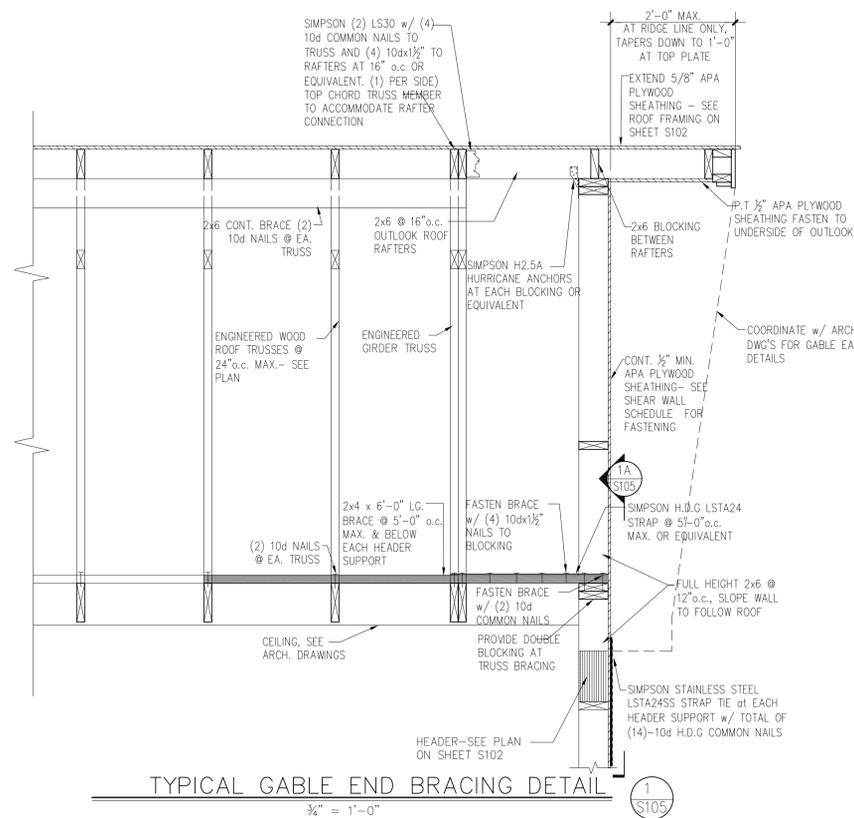
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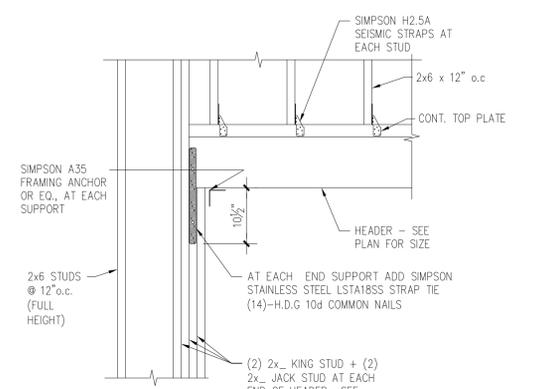
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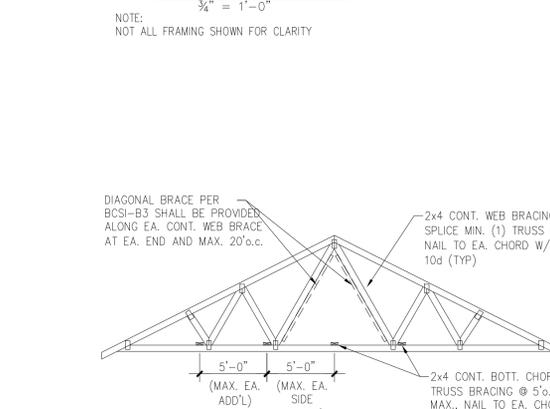
ENVIRONMENTAL ENGINEER:



TYPICAL GABLE END BRACING DETAIL
1/4" = 1'-0"

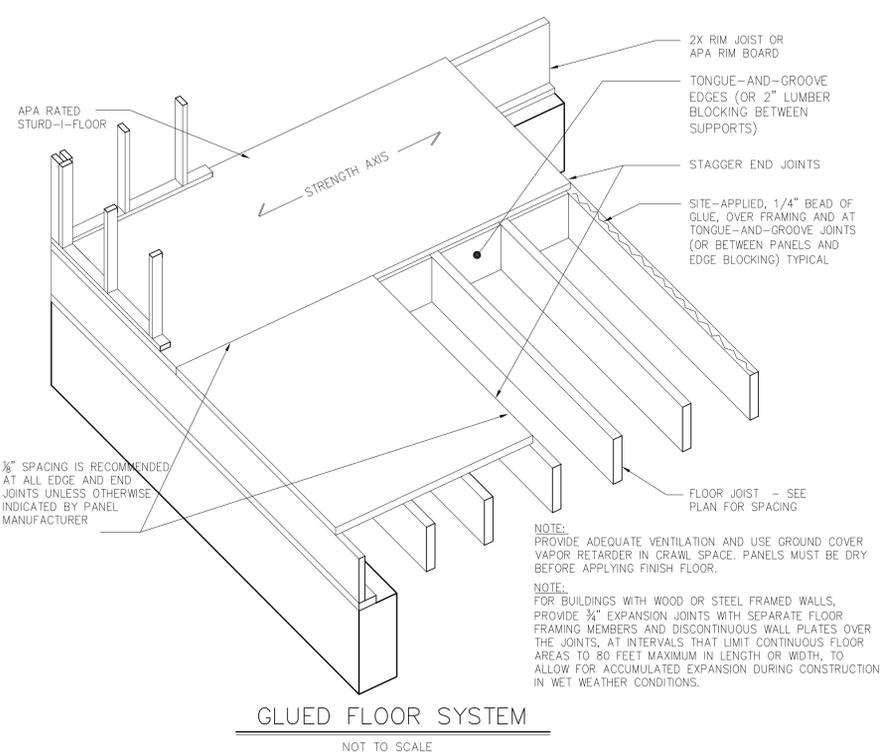


HEADER ANCHORAGE DETAIL AT GABLE END
1/4" = 1'-0"

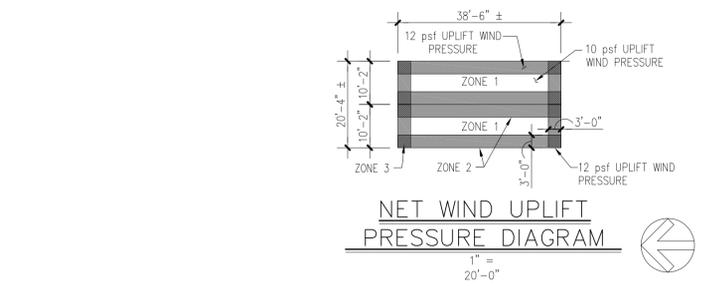


TYPICAL PERMANENT TRUSS BRACING DETAIL
NOT TO SCALE

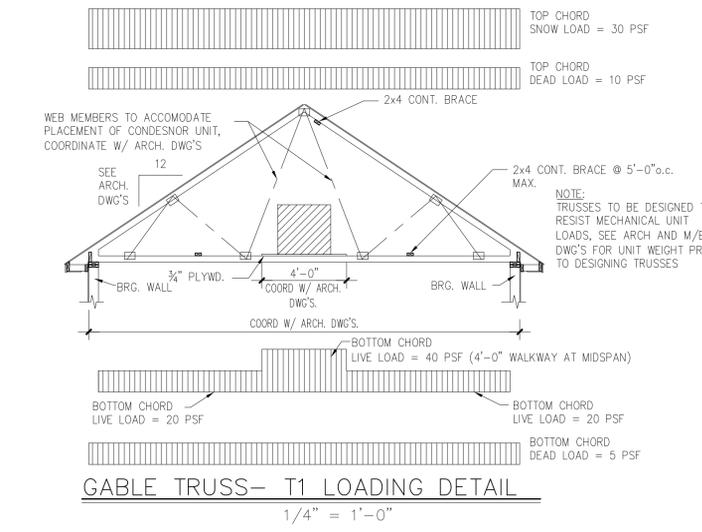
- NOTES:**
- TRUSS BRACING SHOWN MAY NOT REFLECT ALL REQUIRED BRACING (BOTH TEMPORARY AND PERMANENT) THAT IS REQUIRED TO COMPLETE THE TRUSS INSTALLATION. THE CONTRACTOR SHALL BE RESPONSIBLE TO INSTALL BRACING IN ACCORDANCE WITH THESE DRAWINGS, THE TRUSS SHOP DRAWINGS AND THE BCSI MANUAL.
 - THE EXACT QUANTITY OF CONTINUOUS WEB BRACES SHALL BE DETERMINED FROM THE TRUSS SHOP DRAWINGS. A MINIMUM OF 2 CONTINUOUS WEB BRACES SHALL BE PROVIDED BETWEEN TRUSSES ON THE 2 CENTERMOST COMPRESSION MEMBERS WHERE NO CONTINUOUS WEB BRACING IS NOTED ON THE TRUSS SHOP DRAWINGS.
 - TEMPORARY ERECTION BRACING IS NOT SHOWN HERE AND SHALL BE PROVIDED BY THE CONTRACTOR IN ACCORDANCE WITH THE BCSI MANUAL.



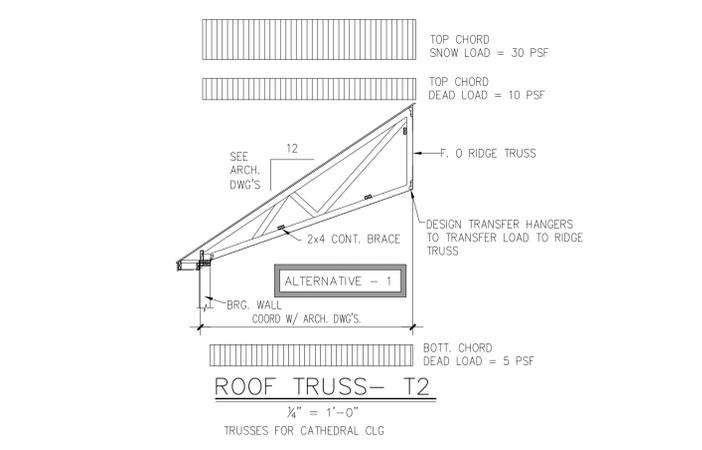
GLUED FLOOR SYSTEM
NOT TO SCALE



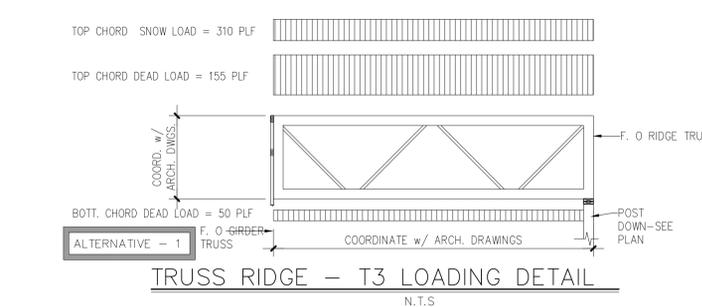
NET WIND UPLIFT PRESSURE DIAGRAM
1" = 20'-0"



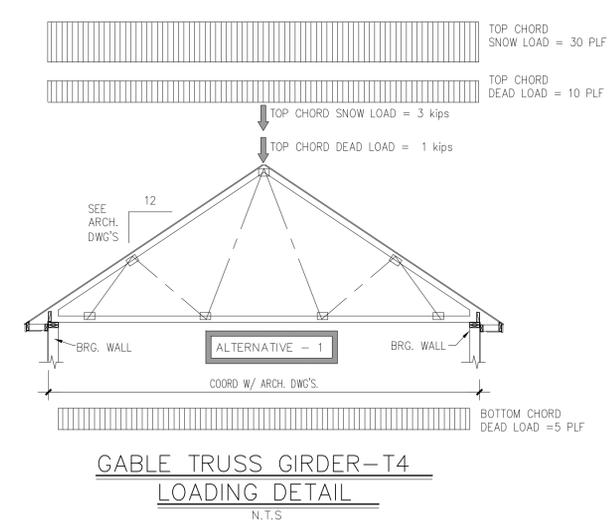
GABLE TRUSS- T1 LOADING DETAIL
1/4" = 1'-0"



ROOF TRUSS- T2
1/4" = 1'-0"



TRUSS RIDGE - T3 LOADING DETAIL
N.T.S.



GABLE TRUSS GIRDER- T4 LOADING DETAIL
N.T.S.

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TRUSS LOADING, TYPICAL DETAILS

PROJECT NO.: 1524-36 SCALE AS NOTED

DRAWING NO.:

S-105

N:\10 PROJECTS\Housing-Residence\1524-36_DRAWINGS\1524-36_15_0708_Structure Drawings\15-110_S1.dwg

GENERAL

- All details shall be considered typical and shall apply to all same and similar conditions.
The Contractor shall field measure and verify all dimensions of the existing building and all dimensions related thereto.
The Contractor shall be responsible for all temporary shoring and bracing required to maintain the structural stability of the building during construction.
All work shall be in accordance with Connecticut State Building Code (CSBC) which includes the 2009 International Residential Code, and the Connecticut 2013 Amendments
The Contractor shall be solely responsible for construction site safety.

DESIGN LOADS

The Structure has been engineered to resist the following design loads in accordance with 2009 IRC Chapter 3 and the Connecticut 2013 Amendments.

- Floor live loads: First Floor - Living 40 psf, Deck 40 psf
Snow load: Ground Snow Load - Pg = 30 psf, Importance Factor - I = 1.0, Exposure Factor - Ce = 1.0, Thermal Factor - Ct = 1.0, Flat Roof Snow Load - Pf = 0.7 x Ce x Ct x I x Pg = 21.0 psf
The roof structure was engineered for a minimum snow load of 30 psf plus snowdrift loads where the low roof abuts the high roof.

- Wind load: Main Wind Force - Resisting System, Basic Wind Speed, (3 sec gust), V = 100 mph, Exposure Classification - C, Importance Factor - I = 1.0, Velocity Pressure Exposure Coefficient, Kz = 0.85, Wind Directionality Factor, Kd = 0.85, Topographical Factor, Kt = 1.0, Product of Internal Pressure Coefficient and Gust Factor, GCpi = +/- 0.18, Gust Effect Factor, G = 0.85, External Pressure Coefficient, Cp = varies, Windward Wall, Cp = 0.80, Leeward Wall, Cp = -0.50, Side Wall, Cp = -0.70, Velocity Pressure, qz = 0.00256 x Kz x Kzt x V^2 x I = 19.93 psf, Design Wind Pressure, p = q x (G x Cp) - qi x (GCpi) use 16 psf

- Earthquake load: Residential structures are exempt.

WIND RESISTING FRAMING:

- Wall Bracing Method: House to be braced by Segmental Shear Walls, the shear walls consist of 1/2" APA Rated Plywood Sheathing, nailed to the wall framing with 10d common wire nails, refer to sheet S103 for nailing spacing.
Braced Wall Lines: the exterior walls are braced wall lines.
Braced Wall Panel length and location: the braced wall panel locations are at all exterior plywood sheathed walls.
All Headers are required to be strapped. Header anchorage detail on drawing S103.
Nailing Spacing and nail type for braced wall panels: the shear walls consist of 1/2" APA Rated Plywood Sheathing, nailed to the wall framing with 10d common wire nails, refer to sheet S103 for nailing spacing.

ENGINEERED LUMBER NOTES:

- Parallel strand lumber, PSL, shall be Wolmanized Paralom as manufactured by Trus Joist, service level 2.
PSL material shall have the following minimum allowable stresses: Flexural stress, Fb = 1,827 psi, Modulus of elasticity, E = 1,460,000 psi, Compression perpendicular to grain, Fc = 368 psi, Compression parallel to grain, Fc|| = 1,508 psi, Horizontal shear, Fv = 197 psi.
Laminated veneer lumber, LVL, shall be Microllam as manufactured by Weyerhaeuser or Equivalent.
LVL material shall have the following minimum allowable stresses: Flexural stress, Fb = 2,600 psi, Modulus of elasticity, E = 1,900,000 psi, Compression perpendicular to grain, Fc = 750 psi, Compression parallel to grain, Fc|| = 2,510 psi, Horizontal shear, Fv = 285 psi, Tension Stress, Ft = 1,555 psi.
Unless otherwise noted on drawings, multiple piles of flush LVL or PSL material shall be bolted together with (2) rows of 5/8 inch diameter, A307 thru-bolts, spaced at 16 inches on center. Bolt holes are to be the same diameter as the bolt, and be located 2 1/2 inches from the top and bottom of the member. Washers should be used under the head and nut of the bolts. Do not tighten bolts to the point of crushing wood fibers. Bolts are to be snug tight. Members noted as dropped shall be connected with (3) rows of 16d common wire nails at 12" on center.

STRUCTURAL STEEL:

- The design complies with the AISC, "MANUAL OF STEEL CONSTRUCTION - ALLOWABLE STRESS DESIGN", Thirteen Edition.
Steel work shall be in accordance with AISC "SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
Welding electrodes shall be E70XX-X and comply with AWS A5.1 and AWS A5.5.
Structural steel: ASTM A36 for angles, plates, and miscellaneous SECTIONS.
Steel hardware: Anchor bolts: ASTM F1554, Carriage bolts: ASTM A307, High strength structural bolts ASTM A325, w/ hexagonal heads Washers: ASTM F436.
All structural steel, clips, and fasteners shall be hot-dip galvanized.
Field apply min. two (2) coats of ZRC to all marred galv. surface.

STRUCTURAL NOTES

TIMBER PILES AND PILE DRIVING:

- All timber pile installation operations shall be supervised by a professional Engineer (Pile Engineer), licensed in the State of Connecticut, and hired by Architect and who has experience with the installation and monitoring of timber piles.
Piles shall be driven to a minimum Allowable Capacity of 20 tons as determined by Engineering News Formula.
Drive the piles straight and true at indicated locations, with deviation from the longitudinal axis of not more than 1/4 inch per foot.
Locate the piles within 3 inches of the positions indicated on the drawings.
Continuously drive each pile to reach the capacity and/or full embedded length called for on the drawings.
Withdraw piles that encounter underground obstructions sufficient to impede pile driving. Redrive as close as possible to original position, subject to review of the Owner. Remove piles which split, broom, break or drive out of line. Drive another pile in its place. Provide and maintain necessary lighting and barriers to adequately assure public safety. Provide adequate safeguards to protect from damage improvements on the work site and on adjacent properties.
Jetting to assist penetration will not be permitted unless accepted by the Structural Engineer of Record.
Where piles are pushed up by pressure from driving of adjacent piles, re-drive as required.
Piles shall be sound Southern Pine with 8 inch minimum tip diameter and shall conform to ASTM D25.
Piles shall be pressure treated with CCA to a retention of 2.0 pcf, in accordance with AWWA C3-81.
Pile driving shall be observed and logged by a Professional Engineer or a Trained Technician Employed by a Professional Engineer.
Pile butts shall be treated with three coats of CCA after cut-off.
Pile driving leads shall be plumb and rigidly fixed. Hanging leads are not acceptable.
Minimum pile embedment to be 15 feet.
The As-Built location of all piles shall be established by a licensed land surveyor. Piles which deviate more than 3 inches from design location or are out of plumb more than 2% of their length shall be reviewed by the Engineer, who shall determine what remedial measures, such as driving additional piles.

- Written installation records shall be obtained for each Timber pile. The records shall include, but are not limited to, the following: a. Project name and location, b. Name of Contractor's foreman and representative who witnessed the installation, c. Date and time of installation, d. Location and/or reference number of each pile, e. Overall depth of installation, f. Any other relevant information relation the installation such as, but not limited to, depth of any obstructions encountered or offset from plan location.

FOR ESTIMATE PURPOSE

- Main piles supporting the house shall be installed to a depth of 25 feet below average elevation 6.0'. The exact embedment lengths shall be verified and recorded in the field by Pile Engineer. For bid purposes, an aggregate lineal footage of piles computed on the basis of the number of piles indicated on the drawings multiplied by an assumed average installed length of pile of 35 feet from tip to cut-off. Final payment for installation length shall be determined using a constant unit price.

ROUGH CARPENTRY

- All framing lumber and plywood shall be clearly marked with a grade stamp.
All wood framing in contact with concrete or masonry shall be ACO preservative treated, or equivalent, in accordance with AWWA Standards.
Keep materials under cover and dry. Protect against exposure to weather and contact with damp wet surfaces. Stack lumber and plywood and other panels; provide air circulation within and around stacks and under temporary coverings including polyethylene and similar material.
Provide lumber with 19% maximum moisture content at time of enclosure for sizes 2" or less in nominal thickness, unless otherwise indicated.
Wall studs: (2x6's, 8' and shorter) provide Douglas Fir-Larch, "stud grade" or better.
For structural framing (2" to 4" thick, 5" and wider), provide Douglas Fir-Larch No. 2 grade or better; except preservative treated lumber shall be Southern Pine No. 2 or better.
All plywood shall be manufactured from a Group 1 or Group 2 species.
Combination Subfloor-Underlayment: APA RATED STURD-I-FLOOR. Exposure Durability Classification: EXPOSURE 1. Span Rating: As required to suit joist spacing indicated or as noted on drawings. Edge Detail: Tongue and groove.
Wall Sheathing: APA RATED SHEATHING. Exposure Durability Classification: EXTERIOR. Span Rating: 12/0, 16/0, 20/0 for stud spacing of 16" or less.

- Fasteners and Anchorage: Provide size, type, material and finish as indicated and as recommended by applicable standards, complying with applicable Federal Specifications for nails, staples, screws, bolts, nuts, washers and anchoring devices. Provide metal hangers and framing anchors of the size and type recommended by the manufacturer for each use including recommended nails.
Where rough carpentry work is exposed to weather, in ground contact, or in area of high relative humidity, provide fasteners and anchorages with a hot-dip zinc coating (ASTM A 153) or 304/316 stainless steel.

ROUGH CARPENTRY CONT.

- Sill Sealer Gaskets: Glass fiber resilient insulation fabricated in strip form for use as a sill sealer; 1" nominal thickness compressible to 1/32"; selected from manufacturer's standard widths to suit width of sill members indicated.
Carefully select all members. Select individual pieces so that knots and obvious defects will not interfere with placing bolts or proper nailing or making connections.
Cut out and discard all defects which will render a piece unable to serve its intended function. Lumber may be rejected by the Engineer, whether or not it has been installed, for excessive warp, twist, bow, crook, mildew, fungus, or mold, as well as for improper cutting and fitting.
Do not shim sills, joists, studs, or other framing components.
Set carpentry work to required levels and lines, with members plumb and true and cut and fitted.
Securely attach carpentry work to substrate by anchoring and fastening as shown and as required by recognized standards.
Countersink nail heads on exposed carpentry work and fill holes.
Use common wire nails or spikes of the dimensions shown or indicated on the nailing schedule, except as otherwise noted. Use finishing nails for finish work. Select fasteners of size that will not penetrate members where opposite side will be exposed to view or will receive finish materials. Make tight connections between members. Install fasteners without splitting of wood; pre-drill as required.
Drill bolt holes 1/16 inch larger in diameter than the bolts being used. Drill straight and true from one side only. Use washers under head and nut. Do not tighten nut to the point of crushing wood fibers.
Lag bolts and wood screws shall be screwed into place and not driven with a hammer.

STUD FRAMING

- Make all studs single length, unspliced, and platform framed, unless notes to be balloon framed on plan.
Unless otherwise shown, use 2x6 studs on exterior walls spaced 16" o.c..
Provide single bottom plate and double-top plates at all walls. Construct corners and intersections with not less than 3 studs. Provide miscellaneous blocking and framing as shown and as required for support of facing materials, fixtures, specialty items and trim.
Provide continuous horizontal blocking row at mid-height of single-story partitions over 8' high, at midpoint of multi-story partitions, and at all horizontal plywood joints.

PLYWOOD INSTALLATION

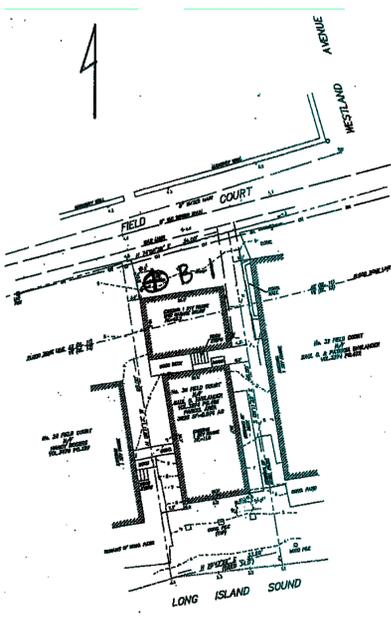
- Place all plywood with face grain perpendicular to supports and continuous over at least two supports. Center joints accurately over supports and stagger the end joints. Install horizontal 2x blocking to match wall framing at all horizontal plywood joints.
Fill and sand edge joints of subflooring-underlayment receiving resilient flooring.
Allow 1/8" spacing at panel ends and 1/4" at panel edges for square edge panels. Allow 1/8" spacing at panel ends and edges for tongue and groove edge panels.

WOOD TRUSSES:

- Lumber: Southern Yellow Pine or Spruce-Pine-Fir, #2 or better for top and bottom chords, #3 or better for web members.
Wood trusses shall be engineered to support the following loads: Roof Trusses: See drawing S105.
Submit to the Architect and Engineer of Record for review, truss shop drawings, a framing plan showing the location of each truss type, and design calculations sealed by a Professional Engineer licensed in the State of Connecticut. The truss fabricator and his Professional Engineer shall assume sole responsibility for the structural adequacy of their trusses.
Trusses shall be designed in accordance with Truss Plate Institute - "Design Specification for Metal Plate Connected Wood Trusses", latest edition.
Trusses shall be erected in accordance with HET "Handling and Erecting Wood Trusses" Truss Plate Institute, latest edition.
Trusses shall be braced in accordance with BWT "Bracing Wood Trusses: Commentary and Recommendations" Truss Plate Institute, latest edition.
Cutting of truss members or alteration of trusses in the field shall not be permitted.
The Special Inspector shall inspect the fabrication and installation of the wood trusses.

WOOD TRUSSES CONT.:

- The Truss Manufacturer shall furnish a Truss Placement Diagram which shall provide the location assumed for each Truss. The Truss Design Drawings shall include as minimum information: a. Slope or depth, span, and spacing; b. Location of all joints; c. Required bearing widths; d. Design loads as applicable; e. Top chord live load (including snow loads); f. Top chord dead load; g. Bottom chord live load; h. Bottom chord dead load; i. Concentrated loads and their points of application; and j. Controlling wind and earthquake loads expressed in units of force per unit area.
Adjustments to lumber and metal connector plate design values for conditions of use;
Each reaction force and direction;
Metal connector plate type, size, thickness or gauge, and the dimensioned location of each metal connector plate except where symmetrically located relative to the joint interface;
Lumber size, species, and grade for each member;
Connection requirements for: (a) Truss to Truss girder; (b) Truss ply to ply and (c) field assembly of Trusses;
Calculated deflection ratio or maximum deflection for live and total load;
Maximum axial compression forces in the Truss members to enable the building designer to design the size, connections, and anchorage of the permanent continuous lateral bracing; and
The appropriate location for continuous lateral permanent bracing of Truss members subject to buckling due to compression forces.
Lumber used shall be identified by grade mark of a lumber inspection bureau or agency approved by the American Lumber Standards Committee, and shall be the size, species, and grade as shown on the Truss Design Drawings, or equivalent as approved by the Truss Designer.
Moisture content of lumber shall be no less than 7% and no greater than 15% at time of manufacturing.
Adjustment of value for duration of load or conditions of use shall be in accordance with NDS.
Metal connector plates shall be manufactured by a Wood Truss Council of America ("WTCA") member plate manufacturer and shall not be less than 0.036 inches in thickness (20 gauge) and shall meet or exceed ASTM A653/A653M grade 33, and galvanized coating shall meet or exceed ASTM A924/924M, coating designation 060. Working stresses in steel are to be applied to effectiveness ratios for plates as determined by test and in accordance with ANSI/TPI 1.
In highly corrosive environments, special applied coatings or stainless steel may be required.
The Truss Manufacturer shall furnish a certified record that WTCA member plate materials comply with steel specifications.
Trusses shall be manufactured to meet the quality requirements of ANSI/TPI 1 and in accordance with the information provided in the final approved Truss Design Drawings.
Trusses shall be handled during manufacturing, delivery and by the Contractor at the job site so as not to be subjected to excessive bending.
Trusses shall be unloaded in a manner so as to minimize lateral strain. Trusses shall be protected from damage that might result from on-site activities and environmental conditions. Trusses shall be handled in such a way so as to prevent toppling when bonding is removed.
Contractor shall be responsible for the handling, installation, and temporary bracing of the Trusses in a good workmanlike manner and in accordance with the recommendations set forth in WTCA/TPI's Building Component Safety Information BCS1-1-03: Guide to Good Practice For Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.
Apparent damage to Trusses, if any, shall be reported to Truss Manufacturer prior to erection.
Trusses shall be set and secured level and plumb, and in correct location. Each Truss shall be held in correct alignment until specified permanent bracing is installed.
Cutting and altering of Trusses is not permitted. If any Truss should become broken, damaged, or altered repair details shall be prepared, signed and sealed by the licensed professional engineer responsible for the truss design. The repair details shall be submitted to the Project Architect and Engineer for review and approval.
Concentrated loads shall not be placed on top of Trusses until all specified bracing has been installed and decking is permanently nailed in place. Specifically avoid stacking full bundles of plywood or other concentrated loads on top of Trusses.
Truss Submittals and any supplementary information provided by the Truss Manufacturer shall be provided by the Contractor to the individual or organization responsible for the installation of the Trusses.
Trusses shall be permanently braced in a manner consistent with good building practices and in accordance with the requirements of the Structural Design Documents. Trusses shall furthermore be anchored or restrained to prevent out-of-plane movement so as to keep all Truss members from simultaneously buckling together in the same direction. Such permanent lateral bracing shall be accomplished by: (a) anchorage to solid end walls; (b) permanent diagonal bracing in the plane of the web members; or (c) other suitable means.
Materials used in temporary and permanent bracing shall be furnished by Contractor.



SOIL BORING LOG

Table with columns for SOILTESTING, INC. (80 DONOVAN RD., OXFORD, CT 06478), CLIENT (Lothrop Associates), SHEET 1 OF 1, HOLE NO. B-1, PROJECT NO. G23-9966-15, PROJECT NAME Englander Residence, LOCATION 30 Field Court Milford, CT, FOREMAN-DRILLER T.Piad, INSPECTOR, GROUND WATER OBSERVATIONS, TYPE HSA, SIZE I.D. 3 1/2", HAMMER W.T. 140#, AT_FT AFTER_HOURS, CORE BAR 140# BIT, OFFSET DATE START 2/13/15, DATE FINISH 2/13/15, SURFACE ELEV., GROUND WATER ELEV., SAMPLE, BLOWS PER IN IN ON SAMPLER (FORCE ON TUBE) 0-6 6-12 12-18, CORE TIME PER FT (MIN), DENSITY OR CONSIST, STRATA CHANGE DEPTH, FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, BEAMS IN ROCK, ETC., and a detailed log of soil boring data from 0 to 45 feet depth.

Lothrop

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100 Pearl Street
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White Plains Rochester Red Bank Hartford

STRUCTURAL ENGINEER:



SURVEYOR & STORM WATER MANAGEMENT:



ENVIRONMENTAL ENGINEER:



Table with columns for ISSUE NO., ISSUE DATE, and DESCRIPTION. Row 1: 1, 25 SEPTEMBER 2015, ISSUED FOR BID.

State Of Connecticut Department Of Housing
505 Hudson Street
Hartford, Connecticut 06106

Application No. 5073
HAZARDOUS MATERIAL ABATEMENT
DEMOLITION OF EXISTING RESIDENCE
CONSTRUCTION OF NEW RESIDENCE
FOR SAUL & PATRICIA ENGLANDER
30 FIELD COURT
MILFORD, CONNECTICUT 06460

STRUCTURAL NOTES AND SOIL BORING LOGS

PROJECT NO.: 1524-36 SCALE AS NOTED

DRAWING NO.:

S-106