

*Owner Occupied and Scattered Site Rehabilitation Programs*

**ADDENDUM ACKNOWLEDGEMENT**

**Addendum #1  
July 15, 2015**

Project # 2319 Property Address: 39 Morehouse Avenue, Milford, CT

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Addendum Description

The addendum modifies the Bid Specification and Drawings prepared by Martinez Couch and Associates, LLC and Consultants to the following particulars;

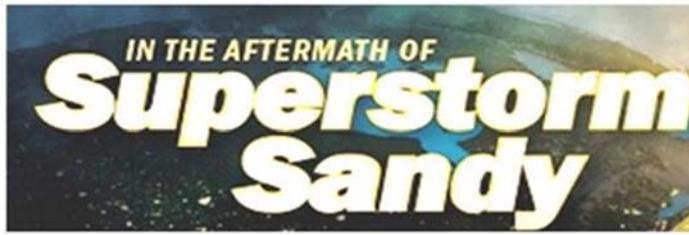
A. Drawings

1. Drawing sheet S-1 and S-4 are replaced with attached sheets. Drawing revisions are noted dated 7/14/2015.

B. General

1. Contractor Bid Walkthrough sign in sheet is attached for bidder's reference.

The bid due date is not extended by this addendum



*Department of Housing*



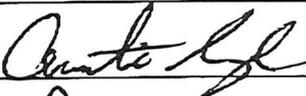
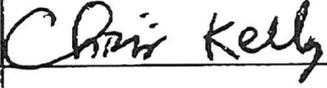
Addendum #1 – Attachment 1

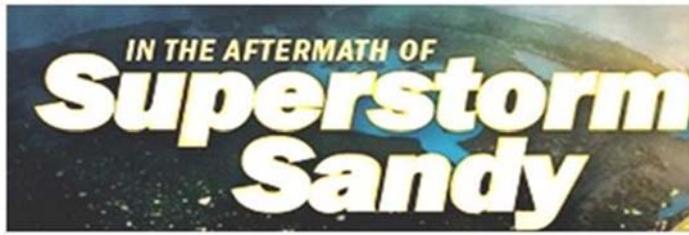
Contractor Bid Walkthrough Attendance Sheet

Stillings

Applicant Number: 2319  
 Date: July 10 15  
 39 Morehouse Mt.

CDBG-DR Owner Occupied Rehabilitation and Rebuilding Program  
 Contractor Walkthrough Sign-In Sheet

Contractor Name	Representative Name	Representative Signature	Phone	Email
MADISON Properties	Ken Esposito		203-218-4141	espokje@aol.com
AUSTIN COPPOLA	BANTON CONSTRUCTION		203-234-2353	rgiammattei@bantonconstruction.com
Ron Means	DSW Homes		86-442-8910	rmeans@dswhomes.com
Len Waitkshis	AIR		203-904-0072	<del>lenwaitkshis@icallair.com</del> lenwaitkshis@icallair.com
<del>MILE SALEY</del> SALEY CONSTRUCTION	MILE SALEY		203-494-2234	SALEY CONSTRUCTION@LIVE.COM
VASE Construction	CHRIS KELLY		203-332-7366	Bids@Vasemanagement.com



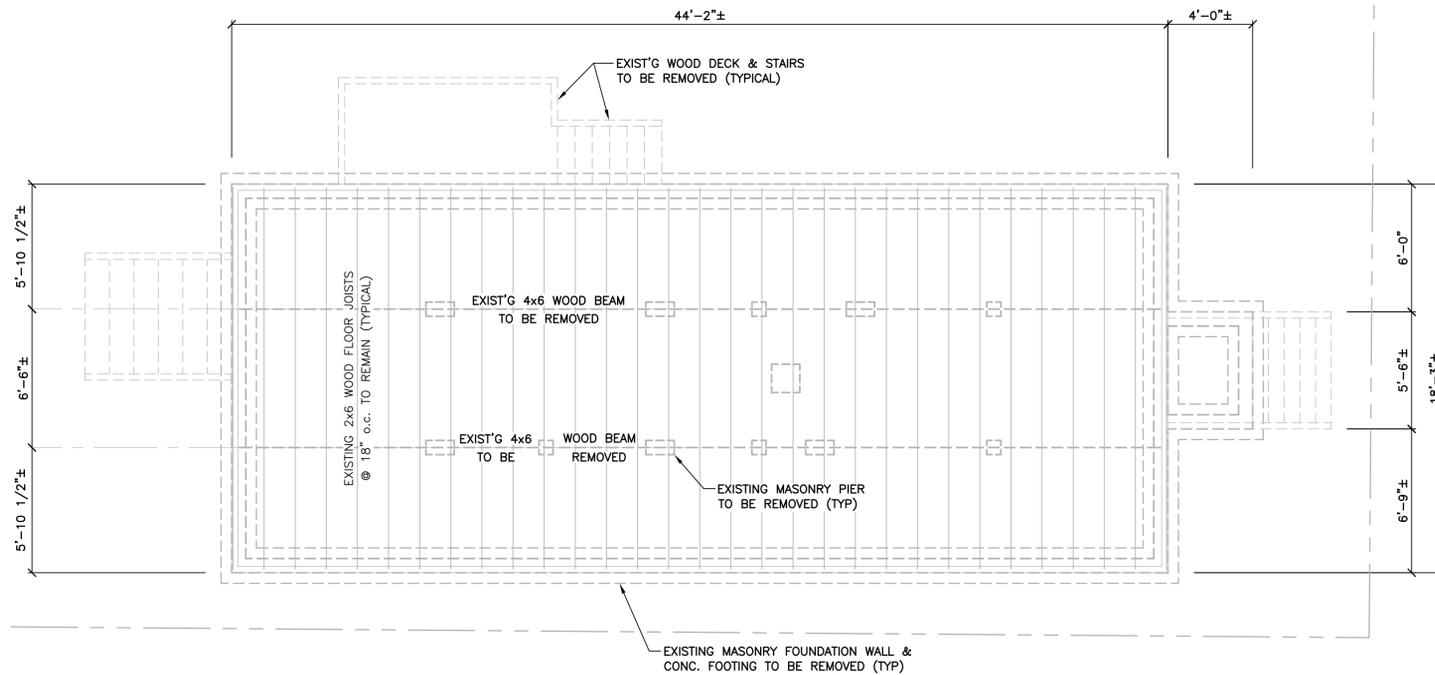
*Department of Housing*



Addendum #1 – Attachment 2

Drawing Sheets – S1 and S-4

EXISTING PROPERTY LINE.  
SEE SURVEY BY OTHERS.



**EXISTING CONDITIONS PLAN**

SCALE: 1/4" = 1'-0"



**NOTES:**

1. THE EXISTING WOOD FRAMED SUPERSTRUCTURE SHALL BE ADEQUATELY SHORED PRIOR TO DEMOLITION OF THE EXISTING FOUNDATION SYSTEM.
2. REFER TO THE ACCOMPANYING DRAWINGS FOR PLANS, DETAILS AND ADDITIONAL REQUIREMENTS RELATED TO THE NEW FOUNDATION AND FRAMING SYSTEM TO SUPPORT THE EXISTING SUPERSTRUCTURE TO BE ELEVATED.
3. CONDITIONS SHOWN ARE BASED SOLELY ON AVAILABLE INFORMATION AND HAVE NOT BEEN INDEPENDENTLY VERIFIED TO BE COMPLETELY ACCURATE.

**DESIGN BASIS**

GOVERNING CODE .....2005 STATE BUILDING CODE +  
2009 & 2013 CONNECTICUT AMENDMENTS

**GENERAL NOTES:**

SHOP DRAWINGS ARE TO BE CHECKED BY THE CONTRACTOR AND BEAR THE CHECKER'S INITIALS BEFORE BEING SUBMITTED TO THE ENGINEER.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES, AND EXISTING CONDITIONS BEFORE PROCEEDING WITH ANY WORK.

IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION.

IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE CONSTRUCTION PROCEDURE AND SEQUENCE, AND TO ENSURE THE SAFETY OF THE EXISTING BUILDING AND ITS COMPONENT PARTS DURING CONSTRUCTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, BRACING, ETC. THAT MAY BE REQUIRED.

WORK THESE DRAWINGS WITH THOSE OF OTHER TRADES FOR LOCATIONS OF OPENINGS, RECESSES, BUILT-IN WORK, ETC.

**FOUNDATION NOTES**

REFER TO GEOTECHNICAL REPORT PREPARED BY DR. CLARENCE WELTI, P.E., P.C., DATED JANUARY 12, 2015 FOR SUBSURFACE PREPARATION & RELATED REQ'MENTS.

ALL NEW FOUNDATIONS SHALL BE SUPPORTED BY NEW HELICAL PULLDOWN PILES MANUFACTURED BY EARTH CONTACT PRODUCTS, LLC, OLATHE KA, OR APPROVED EQUAL.

**HELICAL PILES**

ALL HELICAL PILES SHALL DEVELOP A MINIMUM ALLOWABLE LOAD CARRYING CAPACITY OF 10 TONS (20 KIPS) UNLESS OTHERWISE NOTED.

ALL HELICAL PILES SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF CONNECTICUT. THE PROPOSED HELICAL PILE SHALL HAVE A MINIMUM SHAFT SIZE OF 3 1/2" OUTSIDE DIAMETER x 0.300" WALL THICKNESS, BE FILLED SOLID w/ GROUT AND BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM 123 AND BE SUBMITTED FOR APPROVAL PRIOR TO COMMENCEMENT OF ANY WORK. SUBMITTAL SHALL INCLUDE SIZE, STRENGTH, GALVANIZING, DEPTH, SEAT TYPE AND SIZE.

INSTALL ONE (1) TEST PILE PRIOR TO CONSTRUCTION.

**CONCRETE**

**MATERIALS:**

ALL CONCRETE SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 5,000 PSI AT 28 DAYS. CONTRACTOR TO SUBMIT CONCRETE MIX DESIGN.

ALL DETAILING, FABRICATION, AND ERECTION OF REINFORCING BARS, UNLESS OTHERWISE NOTED, MUST FOLLOW THE LATEST ACI CODE AND LATEST ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES".

ALL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE WITH ASTM A775.

NO CALCIUM CHLORIDE OR ADMIXTURES CONTAINING MORE THAN 1% CHLORIDE BY WEIGHT OF ADMIXTURE SHALL BE USED IN THE CONCRETE.

PROVIDE 5% TO 7% AIR ENTRAINMENT IN ALL CONCRETE.

UNLESS OTHERWISE NOTED, ALL LAP SPLICES SHALL BE CLASS B.

**STRUCTURAL STEEL**

**MATERIALS:**

STRUCTURAL STEEL.....ASTM A572/992  
ANGLES, MISCELLANEOUS PLATES AND BARS.....ASTM A36  
BOLTS.....ASTM A325  
WELDING ELECTRODE.....ASTM E 70

SHOP AND ERECTION DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL STEEL WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. SUBMIT FOUR (4) PRINTS TO THE ENGINEER.

STRUCTURAL STEEL SHALL CONFORM TO THE CURRENT "AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", AND THE "AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".

ALL PERMANENTLY EXPOSED STRUCTURAL STEEL SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH ASTM A123. ALL OTHER STEEL SHALL BE SHOP PRIMED GRAY TO 2 MIL THICKNESS.

ALL WELDING SHALL BE DONE BY A CERTIFIED WELDER IN ACCORDANCE WITH A.W.S. STANDARDS.

ALL STRUCTURAL STEEL, FROM DELIVERY TO THE JOB SITE TO AFTER ERECTION SHALL CONFORM TO ALL REQUIREMENTS OF ASTM A6.

**CONNECTIONS:**

CONNECTIONS SHALL CONFORM TO ALL REQUIREMENTS OF THE "AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", LATEST EDITION, AND THE "SPECIFICATION FOR STRUCTURAL STEEL JOINTS USING ASTM A325 OR A490 BOLTS" APPROVED APRIL 26, 1978.

CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLT ROWS.

SHOP CONNECTIONS SHALL BE WELDED OR HIGH STRENGTH BOLTED.

UNLESS SPECIFICALLY NOTED BOLTS SHALL BE 3/4" TYPE A325-N.

PROVIDE HEAVY HEX NUTS AND WASHERS AT ALL ANCHOR BOLTS.

NO WELDING OR FINAL BOLTING SHALL BE DONE UNTIL AS MUCH OF THE STRUCTURE THAT WILL BE STIFFENED THEREBY HAS BEEN PROPERLY ALIGNED.

MILL BEARING ENDS OF COLUMNS, STIFFENERS, AND OTHER BEARING SURFACES TO TRANSFER LOAD OVER ENTIRE CROSS SECTION.

BOLT HOLES SHALL BE PUNCHED OR DRILLED. FLAME CUT HOLES ARE NOT ACCEPTABLE.

**STRUCTURAL LUMBER**

STRUCTURAL LUMBER USED FOR BRIDGING, BLOCKING AND OTHER MISCELLANEOUS FRAMING SHALL BE NOT LESS THAN NO. 2 GRADE DOUGLAS FIR AND SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE UNIT STRESSES:

EXTREME FIBER IN BENDING..... $F_b = 875$  PSI  
HORIZONTAL SHEAR..... $F_v = 95$  PSI  
TENSION PARALLEL TO GRAIN..... $F_t = 825$  PSI  
COMPRESSION PERPENDICULAR TO GRAIN..... $F_c = 385$  PSI  
COMPRESSION PARALLEL TO GRAIN..... $F_c = 1,050$  PSI  
MODULUS OF ELASTICITY..... $E = 1,700,000$  PSI

MOISTURE CONTENT OF ALL FRAMING LUMBER WHEN DELIVERED TO THE PROJECT SITE SHALL NOT EXCEED 19%.

NAILING AND OTHER FASTENING SHALL BE AS PER TABLE 2304.9.1, FASTENING SCHEDULE, OF THE 2005 STATE OF CONNECTICUT BUILDING CODE.

PLYWOOD WALL SHEATHING SHALL BE NOMINAL 5/8" THICK SQUARE EDGE EXTERIOR GRADE CDX AND CONTAIN (4) INNER PLYS.

PLYWOOD ROOF SHEATHING SHALL BE NOMINAL 3/4" THICK SQUARE EDGE EXTERIOR GRADE.

**ENGINEERED WOOD PRODUCTS**

ELEMENTS. FOLLOWING ERECTION, JOISTS SHALL BE SHEATHED AND PROTECTED TO MAINTAIN STRESS-GRADED MOISTURE CONTENT NOT TO EXCEED 19%.

PARALLAM PLUS PSL WOLMANIZED MEMBERS USED FOR BEAMS AND HEADERS SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE UNIT STRESSES:

EXTREME FIBER IN BENDING..... $F_b = 1,827$  PSI  
HORIZONTAL SHEAR..... $F_v = 197$  PSI  
TENSION PARALLEL TO GRAIN..... $F_t = 1,397$  PSI  
COMPRESSION PERPENDICULAR TO GRAIN..... $F_c = 368$  PSI  
COMPRESSION PARALLEL TO GRAIN..... $F_c = 1,508$  PSI  
MODULUS OF ELASTICITY..... $E = 1,460,000$  PSI



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Fax: (860) 436-4626  
www.martinezcouch.com

**PROJECT 2319**  
39 MOREHOUSE AVENUE  
MILFORD, CONNECTICUT 06460

**THOMAS A. TORRENTI, P.C.**  
Consulting Engineers

316 Dogburn Lane Tel (203) 891-9933  
P.O. Box 1153 Fax (203) 891-8299  
Orange, CT 06477-7153 ttorrenti@optonline.net

NO.	DATE	REVISIONS	BY	CHK	APPV
△	7/14/15	ADDENDUM #1		DEB	TAT
	6/22/15	ISSUED FOR BIDDING		DEB	TAT
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			DATE:		

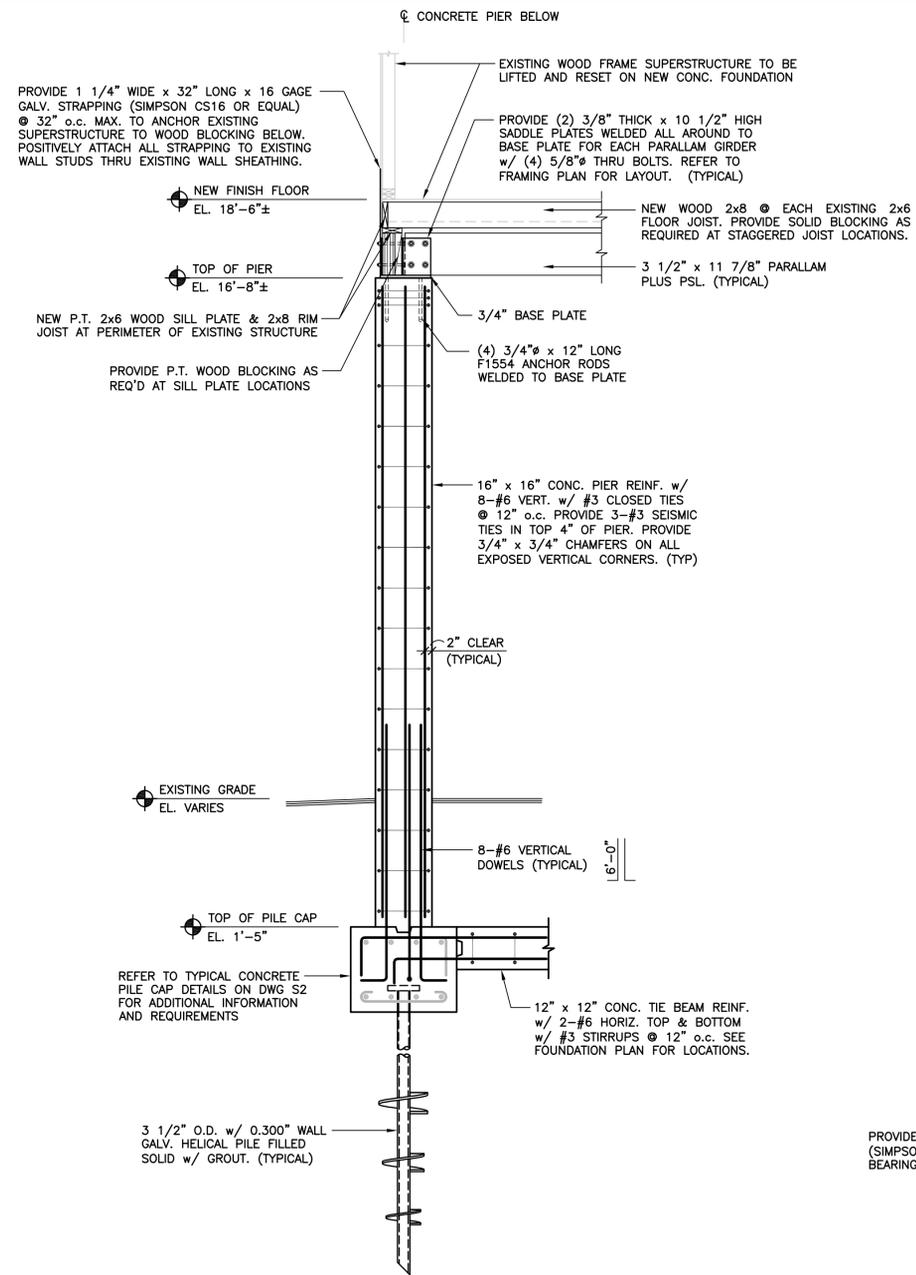
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AND SEAL

THOMAS A. TORRENTI, P.E.

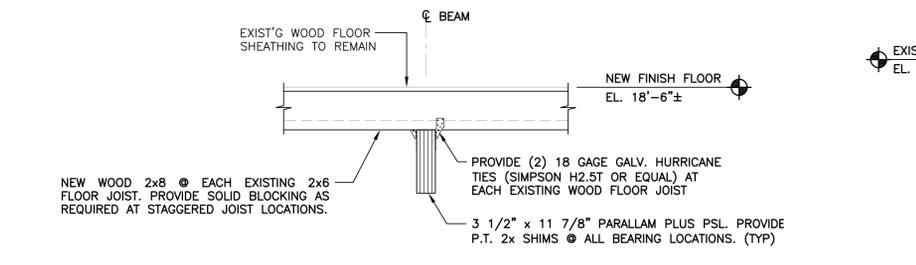
**FOUNDATION DESIGN**

**EXISTING CONDITIONS PLAN  
& STRUCTURAL NOTES**

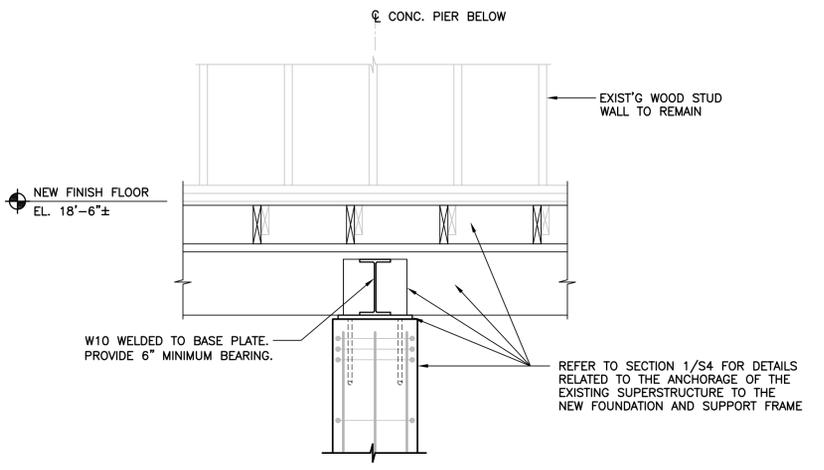
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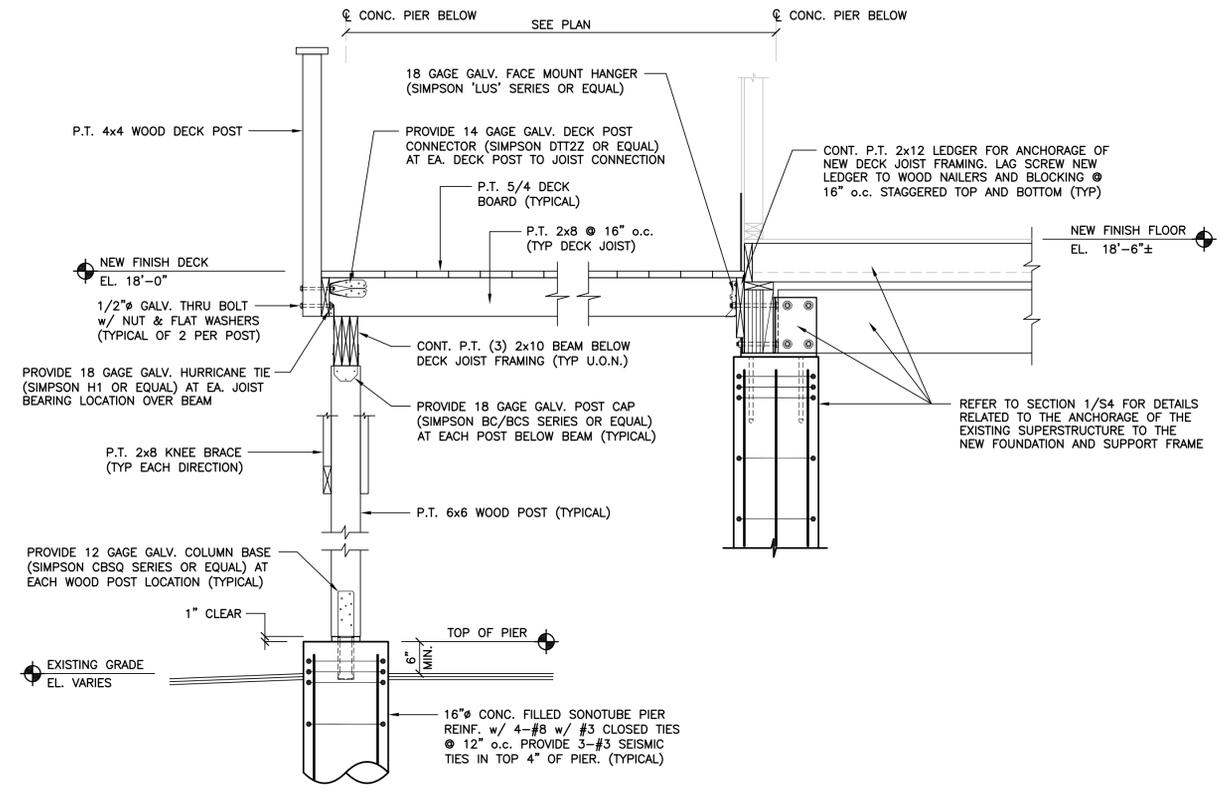
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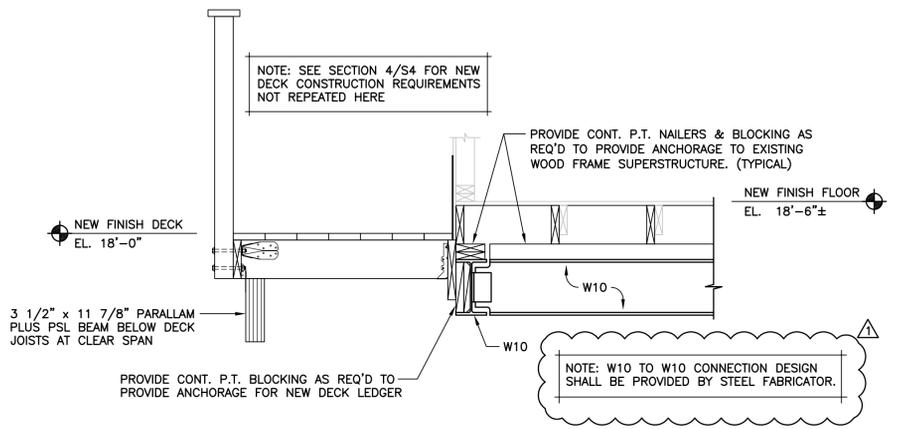
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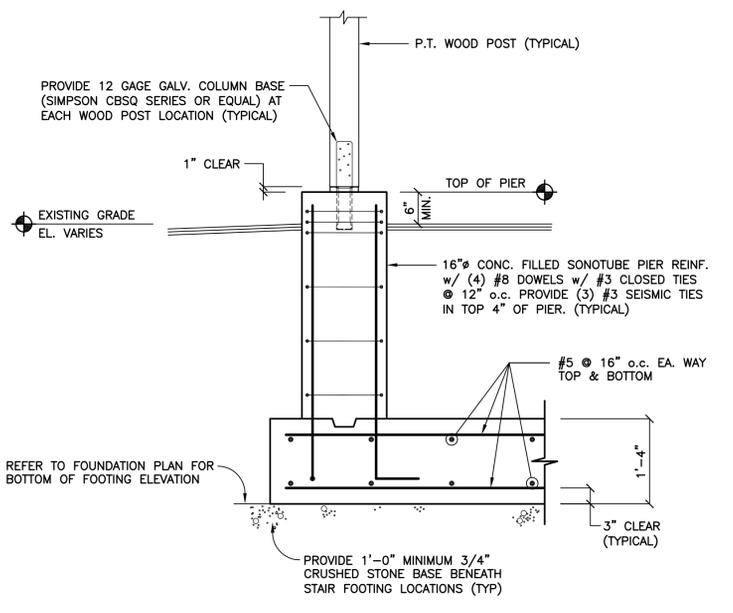
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SECTION 4  
SCALE: 3/4" = 1'-0"



SECTION 5  
SCALE: 3/4" = 1'-0"



SECTION 6 (STAIR FOOTINGS)  
SCALE: 3/4" = 1'-0"



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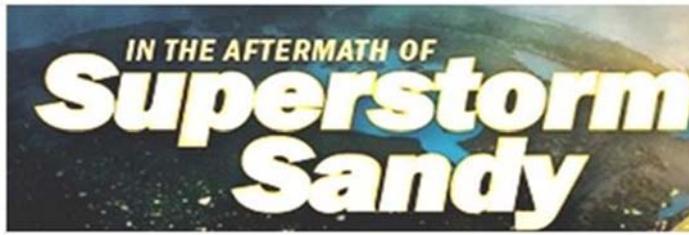
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FOUNDATION DESIGN		
SECTIONS		
JOB NO.	DRAWING NUMBER	SHEET
33-262-2319	1522 S1&S4 ADD #1.dwg	S4



## BID ADDENDUM ACKNOWLEDGMENT

The undersigned acknowledges that he/she have reviewed and acknowledge receipt of this Addendum #1 for Project 2319 – 39 Morehouse Avenue.

Any bids collected which do not contain a signed and executed copy of this page are subject to disqualification.

### BIDDER ACKNOWLEDGEMENT:

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Name of Bidder

Signature

Date

---

Bidder's Title

Bidder's Company