Quisenberry Arcari Architects, LLC
318 Main Street
Farmington, CT 06032

www.qa-architects.com

(860) 677 – 4594 office

(860) 677 – 8534 facsimile

FACSIMILE COVER PAGE

DATE : 07/25/14	JOB # : 2120 QA1346-16
RE: **79 Cooper Avenue Milford, CT**	
(CDBG-DR) & (OORR) Programs	
ADDENDUM #1	

PLEASE COMPLETE SECTION BELOW AND FAX BACK TO 860-677-8534

☐ Received (Addendum #1)

CONTRACTOR : _____

SIGNED : _____ Date: _____

NUMBER OF PAGES INCLUDING COVER PAGE: 13

FROM: Adam Tarfano, AIA

If the attached is incomplete, kindly notify us at once

Community Development Block Grant Disaster Recovery Program (CDBG-DR)
Owner Occupied Rehabilitation and Rebuilding Program (OORR)

#2120 - 79 COOPER AVENUE MILFORD, CT 06460

Addendum # 01
August 15, 2014

GENERAL / CLARIFICATIONS

1. Pre Bid Attendance List (see attached)

2. Specifications: Bid Form (see attached)
   a. Add: Unit Price #4

3. Specifications: 012200 Unit Prices – 3.1 List of Unit Prices – Add "D. Unit Price No. 4: Helical Piles." 1. Unit of Measurement: per Foot.

4. Soil Boring Report and Geotechnical Recommendation (see attached)

5. Clarification: Question during Pre-Bid Walk-through:
   a. Q: Is OSHA 10 certification required for this project?
      A: Yes. Refer to Spec: 011000: 1.12 "Occupational Safety and Health Act (OSHA)" A. 1.

6. Reminder: Bids are due August 28th @ 4:00 pm as stated in the Advertisement for Bids. Any bids received after 4:00pm will be handed back and not accepted.

7. All bidders must be pre-qualified with the State of CT DOH. Bidders who are not pre-qualified may submit the pre-qualification package with their bid. The pre-qualification package may be obtained in hard copy by Geri Rice of CT DOH by phone at (860) 270-8174, by fax at (860) 706-5737, or by email at gerardine.rice@ct.gov.

END OF ADDENDUM #1

Community Development Block Grant Disaster Recovery Program (CDBG-DR)

Owner Occupied Rehabilitation and Rebuilding Program (OORR)

#2120 – 79 COOPER AVE MILFORD, CT

PRE-BID WALK-THROUGH SIGN-IN SHEET

Name	Company	Address	Phone & Fax	Email
Adam Tarfano, AIA	Quisenberry Arcari Architects, LLC	318 Main Street Farmington, CT 06032	860-677-4594 x16	atarfano@qa-architects.com
Jim Russo	J.R Russo, LLC	107 Oakwood Drive, Unit N Glastonbury, CT 06033	860-205-4472	rjames298@aol.com
Bill Martin	Goshen Excavators			wmartin12517@gmail.com
Bruce Small	Vase Management, LLC	360 Fairfield Ave. Suite200 Bridgeport, CT 06604	203-671-7834	crystalprobruce@optonline.net
Connie Pino	Olympus Construction	77 Cherry Street Milford, CT 06460	203-878-1544	jim@olympusconstruction.com
Paul Judson	DSW Homes, LLC	58 River St. Milford, CT 06460	409-939-3344	paul.judson@dswhomes.com
Ken Esposito	Madison Properties, LLC	15 Wintergreen Drive Easton, CT	203-218-4141	espokje@aol.com
Clay Mackham	High Caliber Contracting, LLC	805 East Broadway Milford, CT 06460	203-877-0686	highcal@optonline.net
Doug DiCamillo	DiCamillo Construction, LLC	115 Richards Dr. Monroe, CT 06468	203-459-1104	ddianeHD@aol.com
Robert Warner	Kenneth Warner & Sons, Inc	65-3 North Branford Rd. Branford, CT 06405	203-982-3481 203-481-5991	kwarnernsons@gmail.com robertwarner42@icloud.com

Community Development Block Grant Disaster Recovery Program (CDBG-DR)

Owner Occupied Rehabilitation and Rebuilding Program (OORR)

#2120 – 79 COOPER AVE MILFORD, CT

Jim Pino Jr.	Olympus Construction	77 Cherry Street Milford, CT 06460	203-878-1544	jim@olympusconstruction.com
Peter Cappellino	J.A Rosa Construction, LLC	17 Town Line Road, Wolcott, CT 06716	203-879-3495 203-879-0760	pete@jarosa.com
Tim McFadden	Rockman Millwork	105 Fairview Park Dr. Elmsford, NY 10523	914-434-7463	tim@rockmanmillwork.com
Ron Everett	Vase Management, LLC	360 Fairfield Ave. Suite200 Bridgeport, CT 06604	203-981-3725	vee@vasemanagement.com
Shahnar Jones	Construction Management Systems of America, LLC	4 Fleet St. Suite 4A Waterbury, CT 06704	203-589-1341	shahnar@cmsa.co

BID FORM (Add. #1)

The undersigned, being familiarized with the local conditions affecting the cost of the work and with the Drawings, Specifications, Invitation to Bidders, Instructions to Bidders, General Conditions, Bid Form, Form of Contract and Form of Bonds for Project No. **2120** and Addenda No. ___ and ___ thereto, as prepared by Quisenberry Arcari Architects, LLC at 318 Main Street, Farmington, CT 06032, and on file in the office of DOH, hereby proposes to furnish all permits, labor, materials, tools, equipment, and related items required for the rehabilitation and reconstruction including general construction, site improvements, plumbing, heating, electrical, and finish items for said Project No. **2120** located at **79 Cooper Avenue** in **Milford**, State of Connecticut, all in accordance with the Drawings and Specifications, for the sum of:

_____ Dollars ($ _____)

ALTERNATE PROPOSALS

The undersigned bidder further proposes and agrees that should any or all of the following Alternates be accepted and included in the Contract, the amount of the Base Bid, as heretofore stated, shall be adjusted by the amount stated for each Alternate. All materials and workmanship shall be in strict accordance with the Drawings and Specifications and shall be in-place prices.

Alternates – N/A

Unit Prices

#1	**Plywood Roof Sheathing**	$_____(sqft)
#2	**Plywood Wall Sheathing**	$_____(sqft)
#3	**Gypsum Board, Interior 1/2 inch**	$_____(sqft)
#4	**Helical Piles**	$_____(per ft)

The undersigned agrees to commence the work on a date to be specified in the contract and to complete such work within **150** consecutive calendar days.

The undersigned agrees that if within the period of thirty (30) calendar days after the opening of bids, or when extended to the next work day immediately following said period, notice of the acceptance of this bid shall be mailed, or delivered to him/her at the business address given below, or at any time thereafter before this bid is withdrawn. Quisenberry Arcari Architects will within fifteen (15) calendar days thereafter deliver to DOH, where directed, a contract properly executed in such number of counterparts as may be required by said DOH, on the forms annexed, with such changes therein as shall have been made by the DOH, prior to the time named for delivery of this proposal, together with a 100% Performance Bond of a Surety Company, which Surety must be authorized to transact business in the State of Connecticut, and duly qualified therefore, and in the form constituting part of the Specification and a letter indicating those Small/Minority Business Enterprises that will perform work and/or provide materials, equipment or services as part of the contract.

In submitting this bid, it is understood that the right is reserved by the abovementioned DOH to reject any and all bids; and it is agreed that this bid may not be withdrawn for a period of thirty calendar (30) days from the date of bid opening or until the next work day immediately following said period if such period ends on weekend or a State holiday.

The Geotechnical Department, LLC

Consulting Engineers

41 Blanche Avenue, Demarest, NJ 07627

201-784-4444 • Fax: 201-768-0222

June 2, 2014
Project No. 2048

Quisenberry Arcari Architects, LLC
318 Main Street
Farmington, Connecticut 06032

Attn: Adam Tarfano, AIA

Re: Geotechnical Engineering Report
Proposed Alterations to Raise House
79 Cooper Avenue
Milford, Connecticut

This report is submitted as per our agreement with Soiltesting Inc. and the attached "Geotechnical Limitations." It includes our findings, conclusions and recommendations related to the design and construction for raising the house for future flood protection.

Two (2) test borings was performed by Soiltesting Inc. on May 20, 2013. The location plan and record sheet are attached as part of this report. This information was used in preparation of this report.

Based on our interpretation of field conditions and the scope of the project it was deemed unnecessary to perform laboratory soil tests to assist with the identification of soil and the evaluation of their engineering properties.

Subsurface conditions include soil fill consisting of sand with silt and gravel to depths that range from three (3) feet to four-feet-six-inches (4'6"). Soft organic silt with peat then exists to a depth of about nine (9) feet. Medium dense fine sand with silt continues the soil profile to the maximum depth of exploration, thirty-seven (37) feet.

Water was observed in the borehole at depths of four (4) and five (5) feet at the time the borings were completed.

The following were considered in developing the conclusions and recommendations of this report:

1. A rough sketch plan showing the house plan and boring locations.
2. The proposed work is to raise the existing building foundation for flood protection.
3. Differential settlement should not be greater than three-quarters (3/4) of an inch across the house footprint.
4. Design and construction shall be in accordance with the Connecticut Building Code (Code).

The in-place sand fill and organic layer are not suitable for the support of conventional spread footings. Spread footings bearing on the suitable soil below the fill and organics or on quality compacted soil fill placed after removal of the fill and organics are foundation alternatives. These alternatives, however, are considered impractical due to equipment access constraints, the quantity of excavation and backfill and the need for dewatering the excavation to allow construction.

Deep foundations are deemed a more appropriate foundation alternative. Drilled-in piles could be installed with the tips bearing in the undisturbed medium dense sand system below the in-place fill and organic silt and peat. Driven piles are not an option due to equipment access issues.

The following geotechnical design and construction recommendations are offered:

1. Use drilled-in pile foundations for support of the existing and/or altered house foundation.
2. Tabulated below are design recommendations for several typical pile sections. The capacities refer to the soil/pile interaction, not the structural capacity of the section performing as a column.

Drilled-In Pile Type	Design Capacity (tons)	Installed Capacity* (tons)
Grouted Pipe: 7" diameter	17	20
Auger (Helical)	9	12
Resistance Pier	8	10

** These values consider downdrag on the pile due to settlement of surrounding miscellaneous fill and organics.*

3. The pile tips should penetrate to a minimum depth of fifteen (15) feet below the existing site grades.
4. One (1) drilled-in pile should be load-tested in accordance with the ASTM D1143 Standard prior to installing the production piles.
5. The piles should be delivered to the project under a design-test-install type agreement. The design, testing and installation should be performed under the direction of a licensed engineer with experience with this type of work.
6. The project structural and geotechnical engineer should review the design, test results and installation records.

We trust these recommendations will allow you to complete the design and construction of the alterations.

Very truly yours,
THE GEOTECHNICAL DEPARTMENT, LLC

Thomas H. Otto, P.E.

Attachments: Geotechnical Limitations
Boring Location Plan
Boring Record Sheet

Cc: Soiltesting Inc.

GEOTECHNICAL LIMITATIONS

Explorations

- The analyses and recommendations submitted in this report are based in part upon the data obtained from subsurface explorations. The nature and extent of variations between and apart from these explorations may not become evident until construction. If variations then appear evident, it will be necessary to reevaluate the recommendations of this report.
- The generalized soil profile described in the text is intended to convey trends in subsurface conditions. The boundaries between strata are approximate and idealized and have been developed by interpretations of widely spaced explorations and samples; actual soil transitions are probably more erratic.
- Water level readings have been made in the drill holes at times and under conditions stated on the boring logs. These data have been reviewed and interpretations have been made in the text of this report. However, it must be noted that fluctuations in the level of the groundwater may occur due to variations in rainfall, temperature, and other factors occurring since the time measurements were made. More precise determinations of groundwater levels would require the installation of groundwater observation wells and water level readings taken over an extended period of time.

Review

- In the event that any changes in the nature, design or location of the proposed building are planned, the conclusions and recommendations contained in this report shall not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing by this firm. Further, it is recommended that this firm be provided the opportunity for a general review of final design and specifications in order that earthwork and foundation recommendations may be properly interpreted and implemented in the design and specifications.

Construction

- It is recommended that this firm be retained to provide geotechnical engineering services during construction of the excavation and foundation phases of the work. This is to observe compliance with the design concepts, specifications, and recommendations and to allow design changes in the event that subsurface conditions differ from those anticipated prior to start of construction.

Use of Report

- This report has been prepared for the exclusive use of Quisenberry Arcari Architects, LLC for specific application to construction at 79 Cooper Avenue in Milford, Connecticut in accordance with generally accepted soil and foundation engineering practices. No other warranty, expressed or implied, is made.
- This report is for *design* purposes only and is not sufficient to prepare construction cost estimates or bids.

Phone
(203) 262-9328

Telefax
(203) 264-3414

WHITE PLAINS, N.Y.
(914) 946-4850

SOILTESTING, INC.

90 DONOVAN ROAD - OXFORD, CONN. 06478-1028

GEOTECHNICAL / ENVIRONMENTAL SUBSURFACE INVESTIGATIONS - Test Borings - Core Drilling
Monitoring Wells - Recovery Wells - Direct Push/Probe Sampling
UNDERPINNING - HELICAL PILES - SOIL NAILS

May 28, 2013

Mark Elias
79 Cooper Avenue
Milford, CT 06460
203-962-4845

Attn: Mark Elias

Re: 79 Cooper Avenue
Milford, CT

G103-9416-13

Dear Mr. Elias,

Enclosed are boring logs and location plan for the above referenced project site.

If you have any questions, please do not hesitate to contact us.

Very truly yours,
SOILTESTING, INC.

James A. DeAngelis
President

JAD:lg

SOILTESTING, INC. 55th ANNIVERSARY 1958 OXFORD, CT 2013

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850	CLIENT: **Mark Elias**	SHEET 1 OF 1 HOLE NO. B-1
	PROJECT NO. **G103-9416-13**	
	PROJECT NAME **79 Cooper Avenue**	BORING LOCATIONS per sketch
FOREMAN - DRILLER **Tom**	LOCATION **Milford, CT**	
INSPECTOR	CASING / SAMPLER / CORE BAR TYPE: HSA / SS SIZE I.D.: 4 ¼" / 1 3/8" HAMMER WT.: 140# / BIT HAMMER FALL: 30"	OFFSET DATE START 5/20/13 DATE FINISH 5/20/13 SURFACE ELEV. GROUND WATER ELEV.
GROUND WATER OBSERVATIONS AT 4 FT AFTER 0 HOURS AT FT AFTER HOURS		

DEPTH	CASING BLOWS PER FOOT	SAMPLE NO	Type	PEN	REC	DEPTH @ BOT	BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0-6 / 6-12 / 12-18	CORE TIME PER FT (MIN)	DENSITY OR CONSIST / MOIST	STRATA CHANGE DEPTH / ELEV	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		1	ss	24"	13"	2'0"	1 / 3		moist/wet	0'6"	drk brn SILT, sm FM sand, roots (topsoil)
							5 / 7		loose		brn FM SAND, sm F gravel, lit silt, tr roots
		2	ss	24"	10"	4'0"	8 / 9		wet		brn FM SAND, sm F gravel, lit silt
							10 / 7		compact	4'6"	gry brn FM SAND, sm F gravel, C sand, lit FM silt (Fill)
5		3	ss	24"	18"	6'0"	5 / 2		wet		gry organic SILT, lit peat, clay
							1 / 2		v loose		blk drk brn organic SILT, peat [MH]
		4	ss	24"	22"	8'0"	1 / 2		wet	7'0"	
							1 / 2		v loose		brn FMC SAND, sm organic silt, peat [SM/MH]
		5	ss	24"	24"	10'0"	3 / 4		wet	9'0"	brn FM sand, lit silt tr roots [SM]
10							5 / 6		loose		drk gry FMC sand [SW]
		6	ss	24"	16"	12'0"	2 / 4		wet	11'0"	
							7 / 9		compact		gry VF-F SAND, lit silt [SP]
15		7	ss	24"	20"	17'0"	5 / 6		wet		
							5 / 9		compact		lt brn VF SAND, lit silt [SP]
20		8	ss	24"	17"	22'0"	5 / 6		wet		
							8 / 12		compact		lt brn VF sand, sm silt
25		9	ss	24"	18"	27'0"	7 / 8		wet		
							9 / 12		compact	27'0"	gry SILT, lit clay [ML]
											E.O.B. 27'0"
30											
35											
40											

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO _____FT. USED _____CASING THEN _____ CASING TO _____FT. **HOLE NO. B-1**

A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST

WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE

SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM

PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND =35 - 50% F = FINE

SOILTESTING, INC. 90 DONOVAN RD. OXFORD, CT 06478 CT (203) 262-9328 NY (914) 946-4850	CLIENT: **Mark Elias**	SHEET 1 OF 1 HOLE NO. B-2
	PROJECT NO. **G103-9416-13**	
	PROJECT NAME **79 Cooper Avenue**	BORING LOCATIONS per sketch
FOREMAN - DRILLER **TP/pb**	LOCATION **Milford, CT**	
INSPECTOR	CASING / SAMPLER / CORE BAR	OFFSET
	TYPE: HSA / SS	DATE START 5/20/13
GROUND WATER OBSERVATIONS	SIZE I.D.: 4 ¼" / 1 3/8"	DATE FINISH 5/20/13
AT 5 FT AFTER 0 HOURS	HAMMER WT.: 140# BIT	SURFACE ELEV.
AT __FT AFTER __HOURS	HAMMER FALL: 30"	GROUND WATER ELEV.

DEPTH	CASING BLOWS PER FOOT	SAMPLE NO	Type	PEN	REC	DEPTH @ BOT	BLOWS PER 6 IN ON SAMPLER (FORCE ON TUBE) 0 - 6	6 - 12	12 - 18	CORE TIME PER FT (MIN)	DENSITY OR CONSIST / MOIST	STRATA CHANGE DEPTH / ELEV	FIELD IDENTIFICATION OF SOIL REMARKS INCL. COLOR, LOSS OF WASH WATER, SEAMS IN ROCK, ETC.
		1	ss	24"	14"	2'0"	1	2			moist	0'1"	drk brn SILT, sm MF sand, lit F gravel, roots (topsoil)
							4	8			loose		brn FM SAND, sm silt, lit F gravel,
		2	ss	24"	16"	4'0"	12	16			l moist	3'0"	gry F SAND, sm F gravel, lit M sand (fill)
							18	10			hard		blk drk brn organic SILT, clay, peat [MH]
5		3	ss	24"	24"	6'0"	1	1			moist		
							1	1			v soft		brn gry organic SILT, sm FM sand, peat
		4	ss	24"	22"	8'0"	1	2			moist/wet		
							1	1			soft	8'6"	SAME
		5	ss	24"	13"	10'0"	3	5			moist/wet		brn gry FM SAND, lit F gravel, tr silt [SP]
10							6	7			compact		gry brn FM SAND, lit C sand, tr F gravel
		6	ss	24"	20"	12'0"	5	7			wet	11'6"	
							7	8			compact		gry VF-F SAND, lit silt [SP]
15													
		7	ss	24"	15"	17'0"	4	5			wet		
							6	9			compact		lt brn gry VF-F SAND, lit silt
20													
		8	ss	24"	18"	22'0"	13	14			wet		
							14	12			compact		SAME
25													
		9	ss	24"	13"	27'0"	4	5			wet		
							9	12			compact		gry VF SAND, sm silt
30											wet		
		10	ss	24"	17"	32'0"	5	9			compact		gry SILT [ML]
							10	11					
35													
		11	ss	24"	18"	37'0"	8	10			wet		
							14	16			compact	37'0"	gry SILT
													E.O.B. 37'0"
40													

NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.

GROUND SURFACE TO _____FT. USED _____CASING THEN _____ CASING TO _____FT. **HOLE NO. B-2**

A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST

WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE

SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM

PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND =35 - 50% F = FINE

JOB NO.
G103-9416-13
SOILTESTING, INC.
90 Donovan Road
Oxford, CT 06478