

**STATE OF CT DEPARTMENT OF HOUSING (DOH)
COMMUNITY DEVELOPMENT BLOCK GRANT DISASTER RECOVERY PROGRAM (CDBG-DR)
OWNER-OCCUPIED REHABILITATION AND REBUILDING PROGRAM (OORR)**

For the Rehabilitation/Reconstruction work to:

**Project #1149, Miranda Residence
40 Baxter Drive, Norwalk, CT 06854**

ADDENDUM NUMBER 1

Issue Date: 1/02/15

Addendum Content:

1. Questions and Answers
2. Addendum Acknowledgement to be submitted with contractor's bid proposal

ACKNOWLEDGEMENT of all Addenda must be submitted with the contractor's bid proposal.

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Questions and Answers

(Note: Questions presented *exactly* as received)

1) Question:

“Please advise if there are conflicts with the drawings and the methods typically used in lowering the structure.”

Answer:

Yes, there are conflicts. We’ve shown a proposed lifting steel plan. See attached revised drawings.

2) Question:

“Is there a detail for a beam pocket in the cast in place beams?”

Answer:

Yes. See attached revised drawings.

3) Question:

“Can the concrete reinforcement in the beam be interrupted?”

Answer:

Yes. See attached revised drawings.

4) Question:

“Will there be revised drawings to bid from?”

Answer:

Yes. See attached revised drawings.

5) Question:

“In regards to the bid form on page 7 of the specifications and the allotted time for the project there are concerns for allotting enough time to produce shop drawings, submittals, reviews, ordering materials, fabricating and bending of raw material and then shipping to receive galvanizing and then delivering to the site, concrete cure time ect.. What is the allotted time given for this project?”

Answer:

As part of the bid package each GC is required to produce a project schedule. Each GC will produce said schedule and they will be evaluated as part of the bid process. Note: To save time, rebar can be galvanized before bending and any cracked galvanizing repaired, as needed, in the field.

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6) Question:

“On page SO under concrete notes line 6 states “all reinforcement shall be Galvanized”. Can epoxy coated rebar be used in lieu of hot dip galvanizing the rebar?”

Answer:

NO.

7) Question:

“On page SO Note 22 of the Drawings under “concrete notes continued” It calls out for concrete testing who is responsible for this and should we carry the costs in our bid?”

Answer:

The awarded GC is responsible for all testing. The lab must be approved by the engineer of record.

8) Question:

“On page A-3.0 of the drawings detail “Front Elevation” shows a W8x10 steel I-Beam in what looks to be a dropped header bearing under the floor joist and sill if so I would need the detail for the exterior finishes to bid.”

Answer:

This is a flush header. Please refer to detail 7/S2 of Structural drawings.

9) Question:

“On page S2 of the drawings detail 7/S2 shows a flush header detail in which the exterior siding and sheathing would need to be removed, additional shoring would be needed to bear the weight of the structure while the work in this detail is performed and the bottom flang of the steel I-beam may want to be wider to accept and get more bearing for the joist. This is a more entailed detail either way It really doesn’t matter but please clarify on which drawing to bid by, the Architectual or the Structural drawings?”

Answer:

Refer to Structural drawings.

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ADDENDUM ACKNOWLEDGEMENT

Addendum No.	Issue Date
1.	1/02/15
2.	
3.	
4.	
5.	

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Bidder's Name: _____

Authorized Officer:

(Signature)

(Date)

(Print Name)

(Title/Position)

STATE OF CONNECTICUT DEPARTMENT OF HOUSING

Community Development Block Grant

Disaster Recovery Program

Project: B-13-DS-09-0001

Merritt Construction Services, Inc.

1177 High Ridge Road

Stamford, CT 06905

Applicant Number 1149

40 Baxter Drive

Norwalk Connecticut

CODES:

THE DESIGN AND CONSTRUCTION DOCUMENTS PROVIDED WERE PREPARED IN ACCORDANCE WITH THE FOLLOWING CODES:

THE 2009 INTERNATIONAL RESIDENTIAL CODE AS MODIFIED BY:
2009 AND 2013 AMENDMENT TO THE STATE OF CONNECTICUT BUILDING CODE
2009 INTERNATIONAL ENERGY CONSERVATION CODE
2011 AMENDMENT TO THE 2009 INTERNATIONAL ENERGY CODE
2008 CONNECTICUT STATE FIRE SAFETY CODE AND
2009 AMENDMENT TO THE CONNECTICUT FIRE SAFETY CODE
2003 INTERNATIONAL PLUMBING CODE
2003 INTERNATIONAL MECHANICAL CODE
2011 NATIONAL ELECTRICAL CODE

DRAWING LIST:

T-1.0	TITLE SHEET
SE-1	SITE PLAN
ZLS	ZONING LOCATION SURVEY
A-1.0	FOUNDATION PLAN & DEMOLITION PLAN
A-2.0	FIRST FLOOR PLAN & SECOND FLOOR PLAN
A-3.0	ELEVATIONS
A-4.0	ELEVATIONS
S0-R1	GENERAL STRUCTURAL NOTES
S1	STRUCTURAL FLOOR PLANS
S2-R1	STRUCTURAL DETAILS
DP-1.0	PLUMBING DEMOLITION PLAN
DM-1.0	MECHANICAL DEMOLITION PLAN
DE-1.0	ELECTRICAL DEMOLITION PLAN
P-1.0	PLUMBING PLAN
M-1.0	MECHANICAL PLAN
E-1.0	ELECTRICAL PLAN

Revisions	Date
Bid Set	12.01.14
Bid Set	12.30.14



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THE ARCHITECTURAL WORK
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BY ARIS CRIST AIA

Aris Crist Architects

8 Oak Street West
Greenwich, Connecticut 06830
203 661 0661

APPLICANT No. 1149

40 BAXTER DRIVE
NORWALK CT, 06854

TITLE

Drawn
L.F.O.

Checked

Date
11.25.14

Scale
AS NOTED

Job Number

Sheet

T-1.0

GENERAL NOTES:

- THE WORK SHOWN ON THESE DRAWINGS HAS BEEN DESIGNED IN ACCORDANCE WITH THE STRUCTURAL REQUIREMENTS OF THE 2005 CONNECTICUT STATE RESIDENTIAL BUILDING CODE WHICH IS THE 2009 INTERNATIONAL RESIDENTIAL CODE (IRC), EXCEPT AS AMENDED, ALTERED OR DELETED BY THE PROVISIONS OF THE 2013 CONNECTICUT AMENDMENT.
- THE STRUCTURAL COMPONENTS HAVE BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS:

FLOOR LIVE LOADS:	
ROOMS OTHER THAN SLEEPING ROOMS	40 PSF
SLEEPING ROOMS	30 PSF
STAIRS	40 PSF
DECKS	40 PSF

ATTC LIVE LOAD:

WITH STORAGE, ROOF SLOPE EXCEEDS 3:12	20 PSF
WITHOUT STORAGE, ROOF SLOPE 3:12 OR LESS	10 PSF

ROOF SNOW LOAD:

GROUND SNOW LOAD (Pg)	30 PSF
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WIND DESIGN DATA:

BASIC WIND SPEED (3-SECOND GUST) EXPOSURE:	110 MPH
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- ALL STRUCTURAL WORK SHOWN OR SPECIFIED ON THESE DRAWINGS IS SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER OF RECORD. ASPECTS OF THE WORK FOUND TO BE DEFECTIVE BECAUSE IT DOES NOT MEET THE REQUIREMENTS SHOWN OR SPECIFIED SHALL BE CORRECTED BY THE CONTRACTOR AT NO EXTRA COST TO THE OWNER AS DIRECTED BY THE ENGINEER.
- THIS WORK HAS BEEN DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE CONSTRUCTION HAS BEEN COMPLETED. THE STABILITY OF THE STRUCTURE PRIOR TO COMPLETION IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. THIS RESPONSIBILITY EXTENDS TO ALL ASPECTS OF THE CONSTRUCTION ACTIVITY INCLUDING, BUT NOT LIMITED TO, JOBSITE SAFETY, ERECTION METHODS, ERECTION SEQUENCE, TEMPORARY BRACING AND SHORING, USE OF EQUIPMENT AND SIMILAR CONSTRUCTION PROCEDURES. REVIEW OF CONSTRUCTION BY THE ENGINEER IS FOR CONFORMANCE WITH THE DESIGN ASPECTS ONLY, NOT TO REVIEW THE CONTRACTOR'S CONSTRUCTION PROCEDURES. LACK OF COMMENT ON THE PART OF THE ENGINEER WITH REGARD TO CONSTRUCTION PROCEDURES IS NOT TO BE INTERPRETED AS APPROVAL OF THOSE PROCEDURES.
- SHOP DRAWINGS SUBMITTALS TO THE ENGINEER FOR APPROVAL ARE REQUIRED FOR:
 - CONCRETE REINFORCEMENT
 - STRUCTURAL STEEL
 FABRICATION AND/ OR DELIVERY TO THE SITE OF THESE MATERIALS PRIOR TO RECEIPT OF APPROVAL BY THE ENGINEER IS SOLELY AT THE CONTRACTOR'S OWN RISK.
- SOME DETAILS OF THE WORK MAY BE SHOWN ON THE ARCHITECTURAL DRAWINGS. A CAREFUL REVIEW AND STUDY OF THESE DETAILS ARE NECESSARY BEFORE THE FULL SCOPE OF THE WORK CAN BE COMPREHENDED.
- THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATION, AND ANGLES WITH ARCHITECTURAL DRAWINGS AND EXISTING CONDITIONS BEFORE PROCEEDING WITH ANY WORK.
- DO NOT SCALE DRAWINGS.

FOUNDATION AND EXCAVATION NOTES:

- THE FOUNDATIONS HAVE BEEN DESIGNED TO REST ON INORGANIC, UNDISTURBED SOIL OR COMPACTED GRANULAR FILL HAVING A PRESUMPTIVE BEARING VALUE OF 1500 PSF. SUCH BEARING STRATA IS ANTICIPATED AT THE BOTTOM OF FOOTING ELEVATIONS NOTED ON THE FOUNDATION PLAN. ALL BEARING STRATA SHALL BE REVIEWED BY THE ENGINEER PRIOR TO PLACING CONCRETE IN ORDER TO VERIFY THE PRESUMPTIVE BEARING VALUE.
- IN AREAS REQUIRING FILL, THE FILL MATERIAL SHALL BE A UNIFORMLY GRADED MIXTURE OF SAND AND GRAVEL, WASHING NO LESS THAN 120 PPT DRY DENSITY AFTER COMPACTION IN PLACE. THIS MIXTURE SHALL BE UNIFORMLY GRADED HAVING NO STONE GREATER THAN 3" IN ANY ONE DIMENSION, AND WITH LESS THAN 10% BY WEIGHT, PASSING A #200 SIEVE. THE FILL SHALL BE PLACED IN THIN LIFTS BEFORE COMPACTION. EACH LIFT SHALL BE COMPACTED WITH APPROPRIATE EQUIPMENT TO A MINIMUM OF 95% OF ITS MAXIMUM DENSITY AT OR NEAR OPTIMUM MOISTURE. A SOILS TESTING LAB, HIRED BY THE OWNER, SHALL TEST THE MATERIAL BEFORE AND AFTER COMPACTION WITH THIS SPECIFICATION. NO LIFTS SHALL BE PLACED WHEN WEATHER CONDITIONS ARE SUCH THAT THE MOISTURE CONTENT OF THE FILL CANNOT BE PROPERLY CONTROLLED.
- WITHIN THE PERIMETER OF THE PROPOSED NEW STRUCTURE STRIP THE GROUND SURFACE OF ALL TOPSOIL, ORGANIC AND FILL MATERIAL, COMPACT TOP OF REMAINING EXCAVATED SURFACE.
- THE SLAB-ON-GRADE SUB-BASE SHALL BE CRUSHED STONE PASSING A 2" SIEVE AND WITH LESS THAN 10% BY WEIGHT, PASSING A #100 SIEVE.
- THE BOTTOM OF EXTERIOR FOOTINGS NOT ON SOLID ROCK SHALL BE AT LEAST 3'-6" BELOW FINISHED GRADE. THE SURFACE OF THE SOIL BELOW ALL FOOTINGS SHALL BE MECHANICALLY COMPACTED PRIOR TO SETTING FOOTING FORMS. FOOTINGS ON LEDGE SHALL REST ON BRICK OR CLEAN SOLID ROCK. IF THE SLOPE OF THE ROCK SURFACE EXCEEDS 1 ON 6, THE FOOTING SHALL BE DOWELED TO THE LEDGE WITH 3/4" STEEL RODS DRILLED 10" INTO THE ROCK SURFACE AT 2'-0" O.C.
- DO NOT UNDERMINE EXISTING OR NEWLY PLACED FOUNDATIONS BY EXCAVATING WITHIN A ZONE DIRECTLY BELOW THESE FOUNDATIONS AND EXTENDING DOWN AND OUTWARDS AT A 45° ANGLE.
- PROTECT ALL SOIL UNDER FOUNDATIONS FROM FREEZING DURING CONSTRUCTION. DO NOT POUR CONCRETE ON FROZEN SOIL.
- KEEP FOUNDATION EXCAVATIONS FREE FROM WATER AT ALL TIMES.
- IF STANDING WATER IS PRESENT IN THE FOOTING EXCAVATION, A 4" TO 6" THICK LAYER OF 3/4" CRUSHED STONE SHALL BE COMPACTED INTO THE BOTTOM OF THE EXCAVATION AND DETERIORATING METHODS SHALL BE USED THAT WILL NOT UNDERMINE THE BEARING OF ANY ADJACENT FOOTINGS.
- IN PLACING AND COMPACTING FILL AND BACKFILL MATERIAL, DO NOT DAMAGE NOR DISPLACE CONCRETE WORK ALREADY IN PLACE BY CONTACT FROM COMPACTION MACHINERY, BY SUBJECTING IT TO OVERTURNING FROM HEAVY COMPACTION LOADINGS, OR BY ANY OTHER CAUSE. AT FROST WALLS BRING FILL AGAINST SUCH CONCRETE AT THE SAME RATE AS THE REMAINDER OF FILL, COMPACTING UNIFORMLY ON BOTH SIDES USING HAND OPERATED TAMPERS. IN BASEMENT AND CRAWL SPACE AREAS DO NOT BACKFILL AGAINST WALLS UNTIL THE FLOOR OR ROOF DECK BEARING ON THE WALLS HAS BEEN INSTALLED AND FULLY ATTACHED TO THE TOP OF THE FOUNDATION.
- INVERTS OF FOOTING DRAIN, IF REQUIRED, ARE TO BE SET A MINIMUM OF 2" ABOVE THE BOTTOM OF ADJACENT FOOTINGS.
- USE LEAN CONCRETE (f'c = 1500 PSI) OR CONTROLLED COMPACTED FILL FOR OVER-EXCAVATION OF FOOTINGS.
- WHERE FOOTINGS ARE IN CLOSE PROXIMITY OF SUB-SURFACE PIPING, BOTTOM OF FOOTINGS SHALL BE AT LEAST 8" BELOW ELEVATION OF PIPING, UNLESS OTHERWISE SHOWN ON THE DRAWINGS.
- EXISTING UTILITIES: LOCATE EXISTING UNDERGROUND UTILITIES IN AREAS OF EXCAVATION WORK. PROVIDE ADEQUATE MEANS OF SUPPORT AND PROTECTION DURING EARTHWORK OPERATIONS.

CONCRETE NOTES:

- STRUCTURAL CONCRETE WORK SHALL CONFORM TO ALL THE REQUIREMENTS OF ACI 318-08, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" IN ITS ENTIRETY. CERTAIN PORTIONS OF THIS SPECIFICATION ARE PRESENTED HERE ONLY FOR CLARIFICATION AND THE CONTRACTOR'S CONVENIENCE AND ARE NOT INTENDED TO REPLACE OR AMEND THIS SPECIFICATION.
- CONCRETE SHALL BE NORMAL WEIGHT AND DEVELOP A MINIMUM STRENGTH IN 28 DAYS AS FOLLOWS:

LOCATION	STRENGTH	MAX. WATER/CEMENTITIOUS (W/C) RATIO
FOOTINGS	3000 PSI	0.50
PIERS	4500 PSI	0.45

- PORTLAND CEMENT SHALL BE TYPE I OR TYPE II AND CONFORM TO ASTM C150.
- OTHER CEMENTITIOUS MATERIAL SUCH AS FLYASH OR GROUND GRANULATED BLAST-FURNACE SLAG MAY BE BLENDED WITH CEMENT FOR USE IN THE CONCRETE MIX. FLYASH SHALL CONFORM TO ASTM C618 AND MAY REPLACE CEMENT IF THE FOLLOWING RANGES FOR THE 2 CLASSES OF FLYASH: CLASS C, 20 TO 35% CLASS F, 15 TO 25%. GROUND GRANULATED BLAST-FURNACE SLAG SHALL CONFORM TO ASTM C989 AND MAY NOT EXCEED 50% OF TOTAL WEIGHT OF CEMENTITIOUS MATERIALS.
- COARSE AGGREGATE SHALL BE 3/4" AND CONFORM TO ASTM C33.
- REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. ALL REINFORCEMENT SHALL BE GALVANIZED. THE FOLLOWING WILL APPLY:

THE BAR REINFORCEMENT SHALL BE CLASS 1 GALVANIZED AFTER BAR FABRICATION, IN ACCORDANCE WITH ASTM A767, ZINC-COATED (GALVANIZED) STEEL BARS FOR CONCRETE REINFORCEMENT, WITH AN AVERAGE COATING THICKNESS, FROM A MINIMUM OF 3 TESTS, OF BE 3.5 OZ/SQ FT OR 6 MILS.

ZINC-RICH PAINT USED FOR THE FIELD REPAIR OF GALVANIZED COATINGS SHALL MEET THE FOLLOWING REQUIREMENTS:

 - ONE APPLICATION OF THE MATERIAL SHALL PROVIDE A DRY COATING THICKNESS OF AT LEAST 2.0 MILS.
 - THE APPLIED COATING SHALL PROVIDE BARRIER PROTECTION AND SHALL PREFERABLY BE ANODIC TO STEEL.
 - APPLICATION OF THE COATING MATERIAL SHALL BE POSSIBLE UNDER SHOP OR FIELD CONDITIONS.
 - THE DRIED FILM SHALL HAVE A MINIMUM ZINC DUST CONTENT EQUAL TO 94% (BY WEIGHT).
 - THE BRAND OF MATERIAL USED SHALL BE APPROVED BY THE GALVANIZER, AND SHALL BE COMPATIBLE WITH THE GALVANIZING, AND INERT IN CONCRETE.

FIELD REPAIR - GALVANIZED COATING. THE CONTRACTOR SHALL BE REQUIRED TO FIELD REPAIR ANY DAMAGE TO THE GALVANIZED COATING AND TO REPLACE BARS EXHIBITING SEVERELY DAMAGED COATINGS. THESE REPAIR PROCEDURES ARE ALLOWED ONLY FOR THOSE FIELD REPAIRS DIRECTED BY THE ENGINEER. ALL REPAIRS SHALL BE MADE AT THE CONTRACTOR'S COST. THE GALVANIZED COATING IS TO BE REPAIRED WITH A ZINC-RICH PAINT BY THE FOLLOWING METHOD:

 - CLEAN THE DAMAGED AREA BY WIRE BRUSHING. THE SURFACE SHALL ALSO BE CLEAN, DRY AND FREE OF OIL, GREASE, FLUX RESIDUE, CORROSION PRODUCTS, AND ANY OTHER FOREIGN SUBSTANCE.
 - USING A MINIMUM OF TWO COATS, AND THE METHODS RECOMMENDED BY THE MANUFACTURER OF THE ZINC-RICH PAINT, SPRAY OR BRUSH APPLY THE ZINC-RICH PAINT TO THE AREA IN A MANNER TO ACHIEVE THE APPLICABLE ASTM ADHERENCE AND QUALITY REQUIREMENTS OF THE ORIGINAL COATING, AND A MINIMUM DRY FILM THICKNESS OF 4 MILS.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 WITH A MINIMUM YIELD STRENGTH OF 75 KSI. LAP ONE MESH SIZE AT SIDES AND ENDS, AND WIRE TOGETHER.
- NO WELDING OF REINFORCING WILL BE PERMITTED.
- NO ADMIXTURES ARE PERMITTED WITHOUT THE ENGINEER'S WRITTEN PERMISSION. CONCRETE EXPOSED TO THE WEATHER SHALL CONTAIN 5% ± 1% ENTRAINED AIR.
- GROUT FOR USE UNDER STEEL PLATES SHALL BE CEMENT-BASED, NON-SHRINK, NON-METALLIC GROUT HAVING A MINIMUM 7 DAY STRENGTH OF 5000 PSI, SUCH AS FIVE STAR GROUT MANUFACTURED BY THE U.S. GROUT CORPORATION.
- THE FOLLOWING CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT, UNLESS OTHERWISE NOTED ON PLANS:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3
CONCRETE EXPOSED TO EARTH OR WEATHER:	2
#6 THROUGH #18 BARS:	1 1/2
#5 BAR AND SMALLER:	

CONCRETE NOTES (CONTINUED):

- CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:

SLABS, WALLS, JOISTS:	3/4
#11 BAR AND SMALLER	
BEAMS, COLUMNS = PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1 1/2
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR LIMITING POURS TO MINIMIZE SHRINKAGE CRACKING. IN GENERAL, WALLS SHALL NOT BE POURED IN CONTINUOUS LENGTHS EXCEEDING 30 FEET AND SLABS NOT EXCEEDING 20 FEET WITHOUT CONTROL JOINTS. THE LOCATION AND CONFIGURATION OF JOINTS EXPOSED TO VIEW SHALL BE COORDINATED WITH THE ARCHITECT.
- MINIMUM ANCHOR BOLT REQUIREMENTS FOR ATTACHMENT OF SUPERSTRUCTURE TO FOUNDATION SHALL BE AS FOLLOWS:

CRAWL SPACES, SLABS ON GRADE:	1/2" @ 6'-0" O.C. MAX SPACING
FULL HEIGHT BASEMENT:	3/4" @ 4'-0" O.C. MAX SPACING

 EMBED ANCHOR BOLTS A MINIMUM OF 15" INTO MASONRY. 7" INTO CAST CONCRETE. INSTALL BOLTS WITHIN 1'-0" OF ALL CORNERS AT ALL WALLS. ALL SILL PIECES SHALL HAVE A MINIMUM OF TWO ANCHOR BOLTS.

CONCRETE NOTES (CONTINUED):

- SIZES AND LOCATIONS OF ALL REQUIRED EMBEDDED ITEMS FOR ALL TRADES SUCH AS ANCHOR BOLTS, PIPING SLEEVES, HOLDOWN ANCHORS, ETC. SHALL BE COORDINATED BY THE GENERAL CONTRACTOR WITH OTHER TRADES.
- CONCRETE FORMWORK SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 6, ACI 318. FABRICATION AND PLACEMENT OF REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 7, ACI 318. CONSTRUCTION JOINTS AND EMBEDDED ITEMS, SUCH AS PIPING SLEEVES, SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 6, ACI 318. THE PRODUCTION OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 5, ACI 318.
- THE CONVEYANCE AND PLACEMENT OF THE CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 5, ACI 318. MECHANICAL VIBRATORS ARE TO BE USED TO CONSOLIDATE THE FRESHLY CAST CONCRETE AROUND THE REINFORCING AND AGAINST FORM SURFACES AND TO PREVENT THE FORMATION OF AIR OR STONE FLOCKS, HONEYCOMBS, PITTING OR PLANES OF WEAKNESS. HOWEVER, CARE MUST BE USED TO AVOID OVER VIBRATION THAT CAN LEAD TO AGGREGATE SEGREGATION.
- THE INSTALLATION OF SLABS SHALL CONFORM TO THE REQUIREMENTS OF ACI 302.1R-04. INTERIOR FINISH SLAB SURFACES ARE TO HAVE A CLASS A STEEL TROWEL FINISH. SURFACES OF SLABS FORMING THE SUBSTRATE FOR MUD JOBS ARE TO HAVE A CLASS C SCRATCHED SURFACE. EXTERIOR SLAB SURFACES ARE TO HAVE A CLASS B TOLERANCE WITH THE FINISH AS SPECIFIED ON THE ARCHITECTURAL DRAWINGS.
- THE CURING AND PROTECTION OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 5, ACI 318. CONCRETE SLABS SHALL BE PROTECTED FROM LOSS OF SURFACE MOISTURE FOR NOT LESS THAN 7 DAYS USING A CURING COMPOUND CONFORMING TO ASTM C309 OR CONSTANTLY WETTED BURLAP. COMPOUNDS SHALL BE COMPATIBLE WITH ANY INTENDED FLOORING OVERLAY. DO NOT INSTALL FINISH FLOORING UNTIL SLAB HAS ADEQUATELY DRIED PER THE FLOORING MANUFACTURER'S SPECIFICATIONS.
- COLD WEATHER CONCRETE PLACEMENT: IF COLD WEATHER CONCRETING CONDITIONS EXIST AS DEFINED BY A PERIOD OF MORE THAN THREE DAYS WHEN THE AVERAGE OUTDOOR TEMPERATURE (HIGH + LOW)/2 IS LESS THAN 40° F, THE PROCEDURES OUTLINED IN ACI 306.1 STANDARD SPECIFICATION FOR "COLD WEATHER CONCRETING" SHALL BE UTILIZED.
- HOT WEATHER CONCRETE PLACEMENT: MAINTAIN CONCRETE TEMPERATURE BELOW 90° F AT TIME OF PLACEMENT AND COMPLY WITH ACI 301.
- THE FOLLOWING SUBMITTALS ARE TO BE MADE TO AND APPROVED BY THE ENGINEER PRIOR TO COMMENCING ANY WORK:
 - CONCRETE DESIGN MIX FOR EACH STRENGTH OF CONCRETE REQUIRED ATTESTING THAT THE MIXES CAN ATTAIN THE MINIMUM REQUIRED STRENGTHS IN ACCORDANCE WITH CHAPTER 5, ACI 318.
 - CERTIFICATES OF COMPLIANCE FOR CEMENT, AGGREGATES, AND ADITIVES.
 - SHOP DRAWINGS WITH PLANS, ELEVATIONS, SECTIONS AND BENDING SCHEDULES INDICATING ALL REINFORCING AND ACCESSORIES NEEDED IN ADDITION TO ALL PREVIOUS CONSTRUCTION JOINTS LOCATIONS.
 FABRICATION AND/ OR DELIVERY TO THE SITE OF THESE MATERIALS PRIOR TO RECEIPT OF AND APPROVAL OF THESE SUBMITTALS IS AT THE CONTRACTOR'S OWN RISK.
- A DESIGNATED TESTING LABORATORY SHALL CONDUCT YARDST TEST IN ACCORDANCE WITH THE FOLLOWING PROCEDURES:
 - MAKE ONE STRENGTH TEST FOR EACH 50 CUBIC YARDS OR FRACTION THEREOF FROM EACH MIX DESIGN OF CONCRETE PLACED IN ANY ONE DAY, EXCEPT THAT IN NO CASE SHALL A GIVEN MIX DESIGN BE REPRESENTED BY LESS THAN FIVE TESTS.
 - SECURE COMPOSITE SAMPLES IN ACCORDANCE WITH "METHOD OF SAMPLING FRESH CONCRETE" (ASTM C172). EACH STRENGTH TEST SHALL BE OBTAINED FROM A DIFFERENT BATCH OF CONCRETE ON A REPRESENTATIVE, TRULY RANDOM BASIS. WHEN PUMPING OR PNEUMATIC EQUIPMENT IS USED, SAMPLES SHALL BE TAKEN AT THE DISCHARGE END.
 - MOLD FOUR SPECIMENS FROM EACH SAMPLE IN ACCORDANCE WITH "METHOD OF MAKING AND CURING CONCRETE COMPRESSIVE AND FLEXURE SPECIMENS IN THE FIELD" (ASTM C31), AND CURE UNDER STANDARD MOISTURE AND TEMPERATURE CONDITIONS, IN ACCORDANCE WITH SECTION 7(A) AND 7(B) OF THE ABOVE ASTM METHOD.
 - DETERMINE SLUMP OF THE CONCRETE SAMPLE FOR EACH STRENGTH TEST AND WHENEVER CONSISTENCY OF CONCRETE APPEARS TO VARY USING "METHOD OF TEST OF SLUMP OF PORTLAND CEMENT CONCRETE" (ASTM C43).
 - DETERMINE AIR CONTENT OF NORMAL WEIGHT CONCRETE SAMPLE FOR EACH STRENGTH TEST IN ACCORDANCE WITH EITHER "METHOD OF TEST FOR AIR CONTENT OF FRESHLY MIXED CONCRETE BY PRESSURE METHOD" (ASTM C231) OR "METHOD OF TEST FOR AIR CONTENT OF FRESHLY MIXED CONCRETE BY THE VOLUMETRIC METHOD" (ASTM C173).
 - TEST THREE SPECIMENS: ONE AT SEVEN DAYS, AND TWO AT 28 DAYS IN ACCORDANCE WITH "METHOD OF TEST FOR COMPRESSIVE STRENGTH OF MOLDED CONCRETE CYLINDERS" (ASTM C39). THE 28 DAY TEST RESULT SHALL BE THE AVERAGE OF THE TWO SPECIMENS. IF THE AVERAGE OF THE TWO SPECIMENS IS LESS THAN THE REQUIRED STRENGTH, TEST THE FOURTH SPECIMEN AT 45 DAYS. WHEN HIGH EARLY STRENGTH IS REQUIRED, TWO SPECIMENS SHALL BE TESTED AT SEVEN DAYS.

CONNECTIONS TO EXISTING MASONRY OR CAST-IN-PLACE CONCRETE:

- ALL PROPRIETARY ANCHORING SYSTEMS (EXPANSION, ADHESIVE ANCHORING SYSTEMS, ETC.) TO BE INSTALLED INTO EXISTING CONCRETE AND MASONRY ELEMENTS ARE TO BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS FOR DRILLING AND CLEARING OF HOLES, FOR SPACING AND EDGE DISTANCE REQUIREMENTS, AND FOR THE UTILIZATION OF SUPPLEMENTAL COMPONENTS FOR THE ANCHORING SYSTEMS SUCH AS SCREEN TUBES, DOWLING ADHESIVES, ETC.
- UNLESS NOTED ON PLAN, CONNECTIONS TO EXISTING SOLID CAST-IN-PLACE CONCRETE SHALL BE MADE USING SIMPSON "SET-UP" EPOXY ANCHORING SYSTEM (ADHESIVE ANCHORING SYSTEM, H.E.I. 300-507 ADHESIVE ANCHORING SYSTEM, H.E.I. 300-507 ADHESIVE ANCHORING SYSTEM OR EQUAL AS APPROVED BY THE ENGINEER. SIZE, EMBEDMENT, SPACING, AND EDGE DISTANCES OF ANCHORS AND REINFORCING BARS SHALL BE AS INDICATED ON THE DRAWINGS.
- FOR CONNECTIONS TO EXISTING CONCRETE CONTRACTOR MUST LOCATE THE POSITION OF EXISTING REINFORCING BARS WITH AN E-FLUOR METER OR PLOT HOLES PRIOR TO THE INSTALLATION OF ANCHORS. NOTIFY ENGINEER OF FIELD CONFLICTS PRIOR TO INSTALLATION.

STRUCTURAL STEEL NOTES:

- DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO THE "STEEL CONSTRUCTION MANUAL - THIRTEENTH EDITION" AS ADOPTED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.
- MATERIALS:

WIDE FLANGE SHAPES	ASTM A992, GRADE 50
AMERICAN STANDARD SHAPES, ANGLES, PLATES AND BARS	ASTM A36
BOLTS	ASTM A325
ANCHOR BOLTS	ASTM F1554
WELDING ELECTRODE	ASTM E70XX, LOW HYDROGEN
- ALL WELDING SHALL CONFORM TO AMERICAN WELDING SOCIETY'S AWS D1.1 "STRUCTURAL WELDING CODE-STEEL" CODE FOR ARC AND GAS WELDING AND BE PERFORMED BY A CERTIFIED WELDER IN ACCORDANCE WITH A.W.S. STANDARDS.
- ALL FIELD WELDING IS TO BE VISUAL INSPECTED, UNLESS OTHERWISE NOTED, BY AN A.W.S. CERTIFIED WELD INSPECTOR. REPORTS ARE TO BE SENT TO THE ARCHITECT, ENGINEER, AND OWNER IN A TIMELY MANNER.
- STEEL SHALL BE ERECTED TO A TOLERANCE OF NOT MORE THAN 1/4" IN 10'-0" OUT-OF-PLUMB, NOR 1/8" FROM THE REQUIRED ELEVATION.
- ALL STEEL MEMBERS AND BOLTING EXPOSED TO WEATHER SHALL BE CLEANED IN ACCORDANCE WITH THE STEEL STRUCTURES PAINTING COUNCIL SPECIFICATION SP-6 FOR COMMERCIAL BLAST CLEANED AND HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 AND ASTM A153. MINIMUM ACCEPTABLE ZINC COATING WEIGHT SHALL BE 2 OZ./SQ. FT.
- EXISTING STEEL SURFACES TO RECEIVE FIELD WELDS SHALL BE THOROUGHLY CLEANED AND FREE FROM PAINT, RUST, GREASE, ETC.
- CERTIFICATES OF COMPLIANCE SHALL BE SUBMITTED TO THE ENGINEER FOR STRUCTURAL STEEL, BOLTS, NUTS, WASHERS, AND WELD FIELD MATERIAL PRIOR TO THE FABRICATION OF ANY STEEL.

GENERAL WOOD NOTES:

- WOOD DESIGN IS BASED ON THE AF&PA NDS-05 "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION WITH THE 2005 SUPPLEMENT".
- STUD BEARING WALLS, SHEARWALLS, AND ROOF/ FLOOR DECKS SHALL BE FRAMED WITH THE MEMBER SIZES AND/ OR TYPES AT THE SPACINGS SHOWN ON PLAN. THE CONTRACTOR SHALL COORDINATE LOCATIONS OF ALL PLUMBING PIPING, HVAC DUCTING AND RECESSED LIGHTING FIXTURES, ETC. PRIOR TO LAYOUT TO MINIMIZE INTERFERENCE THAT MAY REQUIRE THE ALTERING OR STRENGTHENING OF THE INSTALLED FRAMING.
- ALL WOOD FRAMING IS TO BE STORED ON SITE ABOVE THE GROUND ON "STICKERS" INDOORS OR UNDER TARPS WITH ADEQUATE CLEARANCES TO ALLOW AIR CIRCULATION.
- WALLS SHALL BE INSTALLED STRAIGHT AND PLUMB. FLOORS SHALL BE INSTALLED LEVEL AT THE PROPER ELEVATION. ROOFS SHALL BE INSTALLED AT THE PITCHES INDICATED ON THE ARCHITECTURAL DRAWINGS.
- JOISTS AND RAFTERS SHALL BE INSTALLED DIRECTLY OVER BEARING STUDS UNLESS OTHERWISE DETAILED.
- JOISTS AND RAFTERS SHALL BE SUPPORTED LATERALLY AT EACH SUPPORT BY FULL DEPTH SOLID BLOCKING, EXCEPT WHERE JOISTS ARE SUPPORTED BY A FLUSH HEADER OR NAILED TO A RIM JOIST.
- UNLESS NOTED ON PLAN, PROVIDE A MINIMUM OF TWO STUDS AT EACH END OF ALL FLUSH FRAMED HEADERS OR BEAMS. UNLESS NOTED ON PLAN, PROVIDE ONE JACK STUD AND ONE FULL KING STUD AT EACH END OF ALL DROPPED HEADERS OR BEAMS. POSTS SHALL BE SOLIDY BLOCKED THROUGH ALL INTERVENING FRAMED BEAMS DOWN TO SUPPORTING GIRDER/ BEAMS OR TOP OF FOUNDATION.
- FLUSH FRAMED CONNECTIONS SHALL BE MADE WITH PREFABRICATED GALVANIZED STEEL HANGERS MADE BY SIMPSON STRONG-TIE COMPANY, INC. OR BY UNITED STEEL PRODUCTS COMPANY (USP) OF WIDTH AND DEPTH APPROPRIATE FOR THE SUPPORTED MEMBER. INSTALL WITH THE TYPE AND QUANTITY OF FASTENERS RECOMMENDED BY THE MANUFACTURER. PREFABRICATED STEEL HANGERS USED IN CONTACT WITH PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 OR STAINLESS STEEL, TYPE 316, OR HAVE A "TRIPLE ZINC" (ASTM 0185) COATING. FASTENERS IN CONTACT WITH PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153 OR STAINLESS STEEL, TYPE 316. DO NOT MIX STAINLESS STEEL AND GALVANIZED FASTENERS AND CONNECTORS.
- CONTRACTOR SHALL CHOOSE METAL CONNECTOR (SIMPSON, USP, OR APPROVED EQUAL) BASED ON MEMBER REACTIONS SHOWN ON THE DRAWINGS, UNLESS OTHERWISE NOTED. CONTRACTOR TO PROVIDE PRODUCT DATA TO THE ENGINEER FOR APPROVAL.
- STRUCTURAL WOOD FRAMING USED IN EXTERIOR APPLICATIONS OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE SOUTHERN YELLOW PINE NO. 2 OR BETTER, ACQ (ALKALINE COPPER QUATERNARY) OR CA (COPPER AZOLE) PRESERVATIVE PRESSURE TREATED WOOD WITH A RETENTION APPROPRIATE FOR END USE.
- BUILT-UP MEMBERS OF THREE PILES OR LESS SHALL HAVE ADJACENT PILES NAILED TOGETHER WITH TWO ROWS OF NAILS AT 12" O.C. (10# COMMON NAILS FOR 1-1/2" PILES, 12# COMMON NAILS FOR 1-3/4" PILES). BUILT-UP MEMBERS OF MORE THAN 3 PILES SHALL BE ASSEMBLED WITH 1/2" THRU BOLTS AT 16" O.C. STAGGERED UP AND DOWN WITH 2" CLEARANCE AT TOP AND BOTTOM EDGES.
- EXTERIOR END WALLS OF CATHEDRAL CEILING SPACES SHALL BE FRAMED WITH STUDS RUNNING CONTINUOUSLY (NOT SPLICED) FROM FLOOR TO ROOF, ADDITIONAL FRAMING MAY BE NECESSARY. SEE PLANS.

DIMENSIONED WOOD FRAMING NOTES:

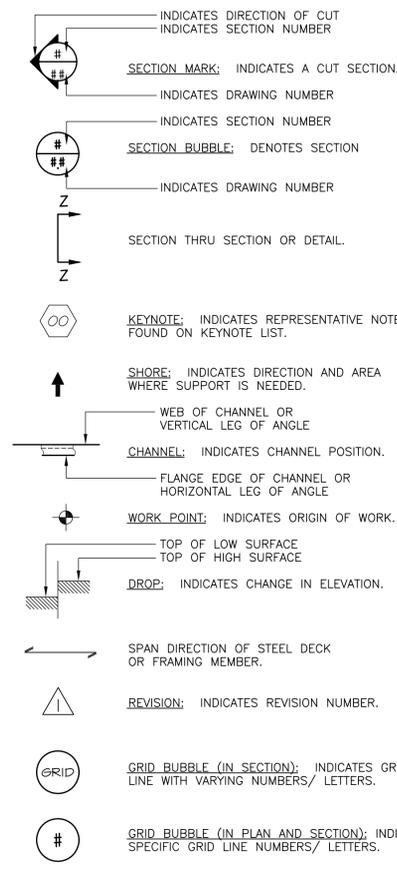
- THE STRUCTURAL WOOD STRESS GRADE STAMPED LUMBER SHALL BE GRADED AS FOLLOWS:

JOISTS, RAFTERS, STUDS:	DOUGLAS FIR-LARCH OR DOUGLAS FIR-LARCH (NORTH), NO. 2
	F5 (GRADE) = 850 PSI,
	E = 1,600,000 PSI
- THE DESIGN OF THE DIMENSIONAL LUMBER MEMBERS AND THEIR CONNECTIONS IS BASED ON THE LUMBER HAVING A MOISTURE CONTENT AT THE TIME OF INSTALLATION OF 19% OR LESS.
- JOISTS OR RAFTERS ARE TO BE INSTALLED WITH "CROWN" UP (I.E. POSITIVE CAMBER) AND WITHIN 1/2" OF STRAIGHT, END-TO-END ALIGNMENT.
- SEVERELY DISTORTED (TWISTED, BOWED, CUPPED, CHECKED, ETC.) LUMBER SHALL NOT BE USED.
- NOTCHES IN THE TOP OR BOTTOM OF DIMENSIONED LUMBER JOISTS OR RAFTERS SHALL NOT EXCEED ONE-SIXTH THE MEMBER DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE THIRD OF THE SPAN, UNLESS OTHERWISE NOTED ON PLANS. END NOTCHES SHALL NOT EXCEED ONE-FOURTH THE MEMBER DEPTH, UNLESS OTHERWISE NOTED ON PLANS. BORED HOLES SHALL NOT BE WITHIN 2" OF THE TOP AND BOTTOM OF THE MEMBER AND THEIR DIAMETER SHALL NOT EXCEED ONE-THIRD THE MEMBER DEPTH, UNLESS OTHERWISE NOTED ON PLANS.

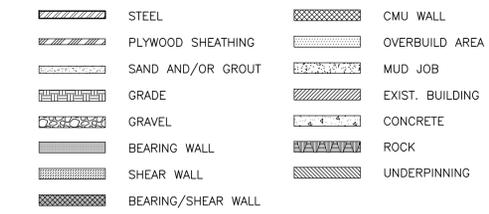
WOOD FASTENERS NOTES:

- BOLTS SHALL CONFORM TO ASTM A307 OR ASTM A36.
- LAG AND WOOD SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.6.1-19.81.
- ALL FASTENERS USED IN CONTACT WITH PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153 OR STAINLESS STEEL, TYPE 316. DO NOT MIX STAINLESS STEEL AND GALVANIZED FASTENERS AND CONNECTORS.
- BORED LEAD HOLES FOR FASTENERS SHALL BE AS FOLLOWS:
 - WOOD SCREWS - LEAD HOLE DIAMETER EQUALS 7/8 OF UNTHREADED SHANK DIAMETER IN CONNECTED WOOD PART AND 7/8 OF DIAMETER AT ROOT OF THREAD IN WOOD RECEIVING THREAD.
 - LAG SCREWS - LEAD HOLE DIAMETER EQUALS SHANK DIAMETER FOR EXTENT OF UNTHREADED SHANK, AND 60% OF SHANK DIAMETER FOR THREADED PORTION OF SHANK.
 - THRU BOLTS - LEAD HOLE DIAMETER 1/32" TO 1/16" LARGER THAN NOMINAL BOLT DIAMETER.
- INSERT THREADED SCREW TYPE FASTENERS BY TURNING WITH SCREWDRIVER OR WRENCH. DO NOT DRIVE BY HAMMERING. FACILITATE INSTALLATION BY PLACING SOAP OR OTHER LUBRICANT ON THREADS.
- PROVIDE STANDARD ROUND WASHERS UNDER THE HEADS OF ALL THRU BOLTS AND LAG SCREWS AND UNDER ALL NUTS UNLESS OTHERWISE INDICATED ON THE PLANS. TIGHTEN FASTENERS WITHOUT CRUSHING WOOD FIBERS UNDER WASHERS.

GENERAL LEGEND



HATCH PATTERNS:



Revisions	Date
△ UPDATE NOTES	12/31/14

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GENERAL NOTES

JM	Drawn
GCF	Checked
	Date
12.31.14	Scale
AS NOTED	Job Number
13288.00	Sheet
S0-R1	

Revisions	Date
△ UPDATE DETAILS	12/31/14



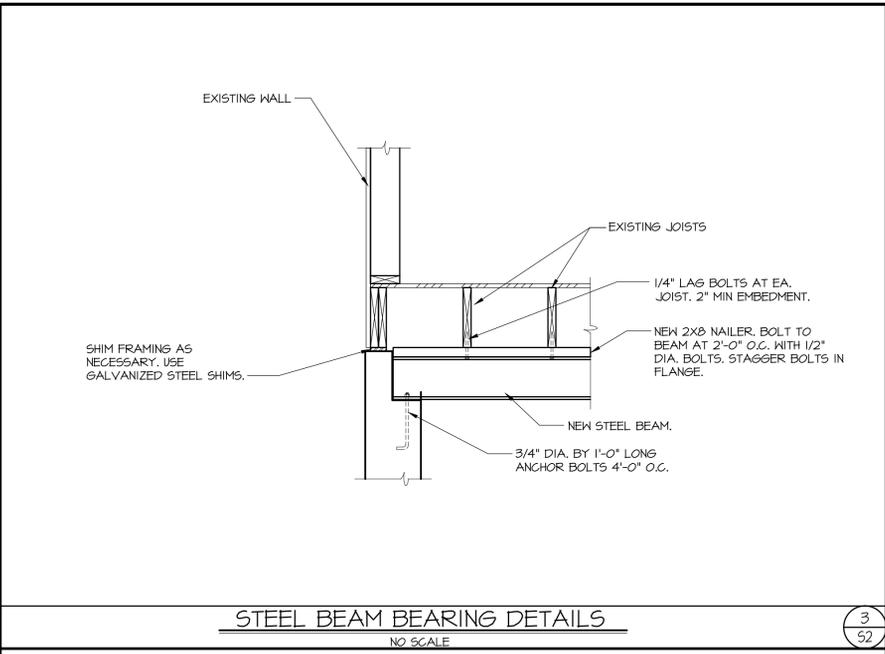
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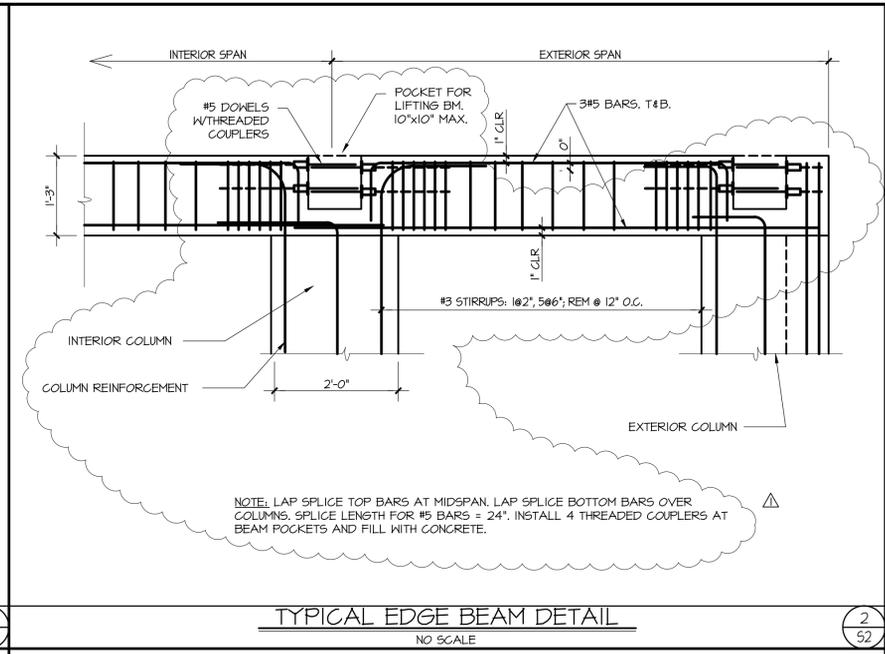
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 DETAILS - NEW FOUNDATION AND REPAIRS

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Checked	GCF
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Scale	AS NOTED
Job Number	13288.00
Sheet	

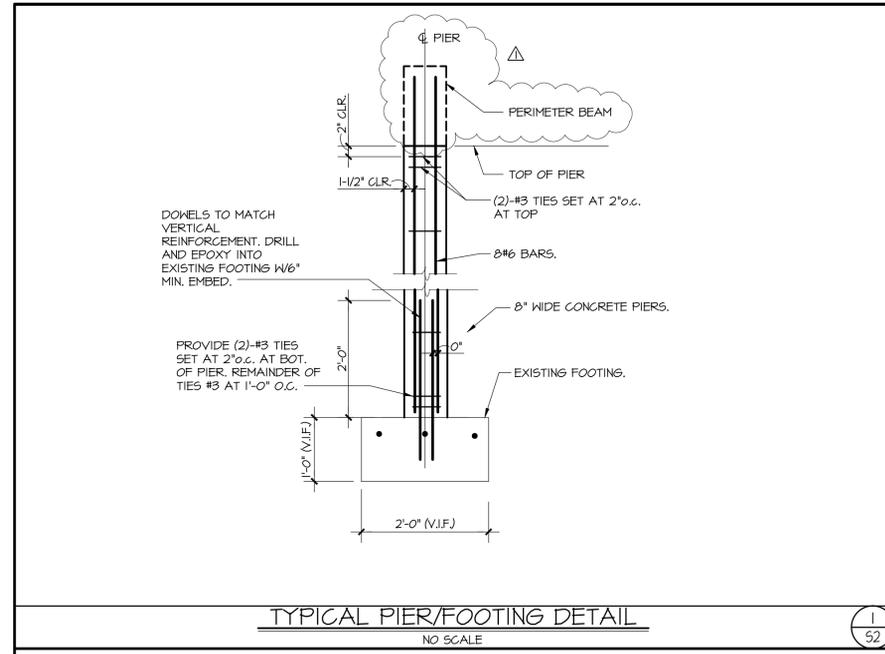
S2-R1



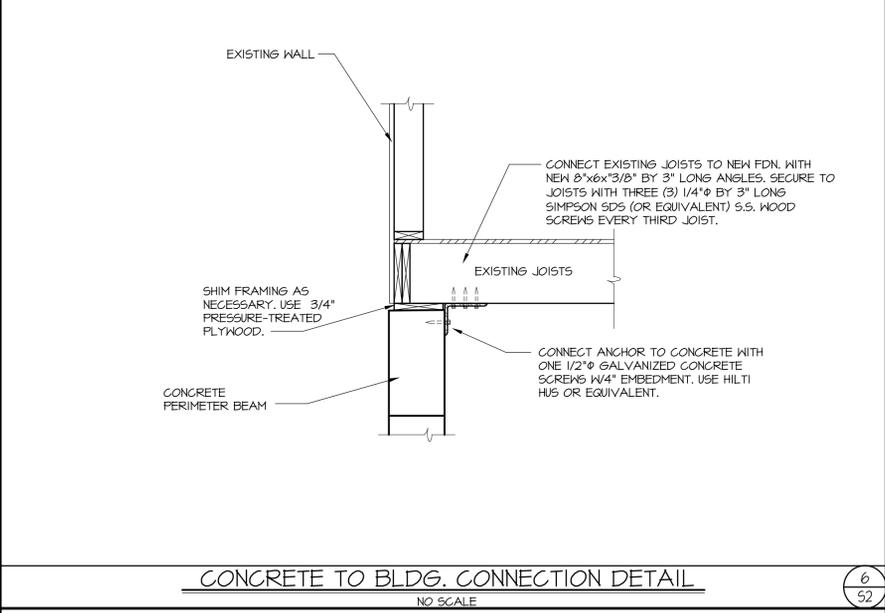
STEEL BEAM BEARING DETAILS
NO SCALE



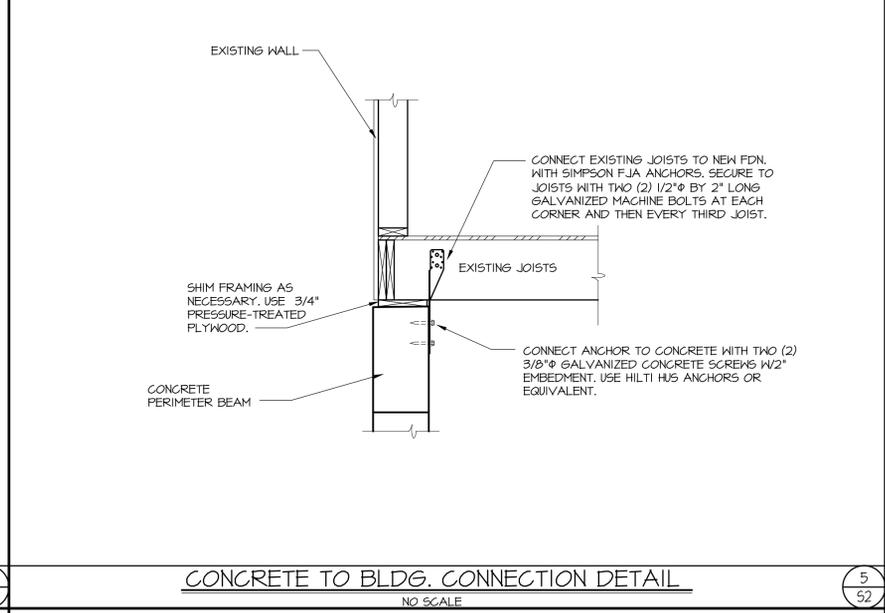
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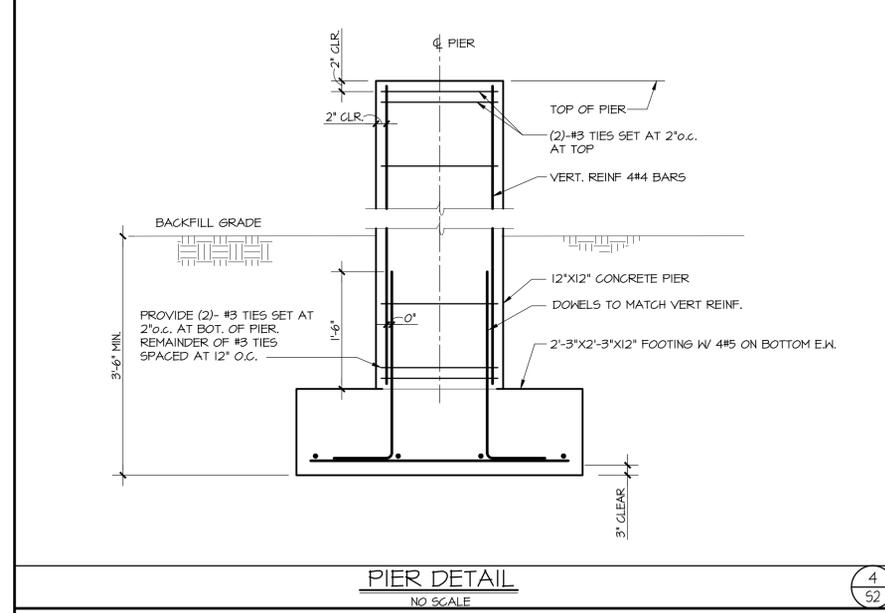
TYPICAL PIER/FOOTING DETAIL
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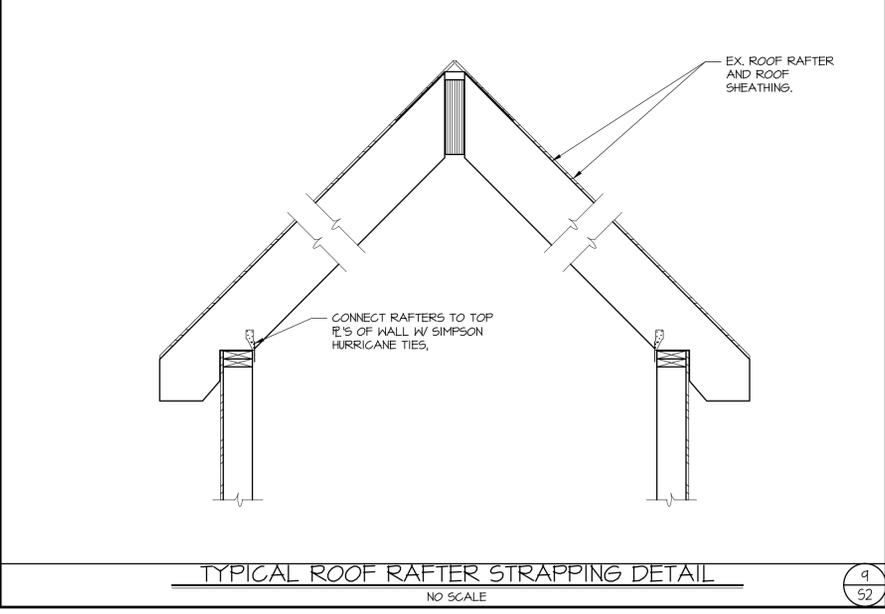
CONCRETE TO BLDG. CONNECTION DETAIL
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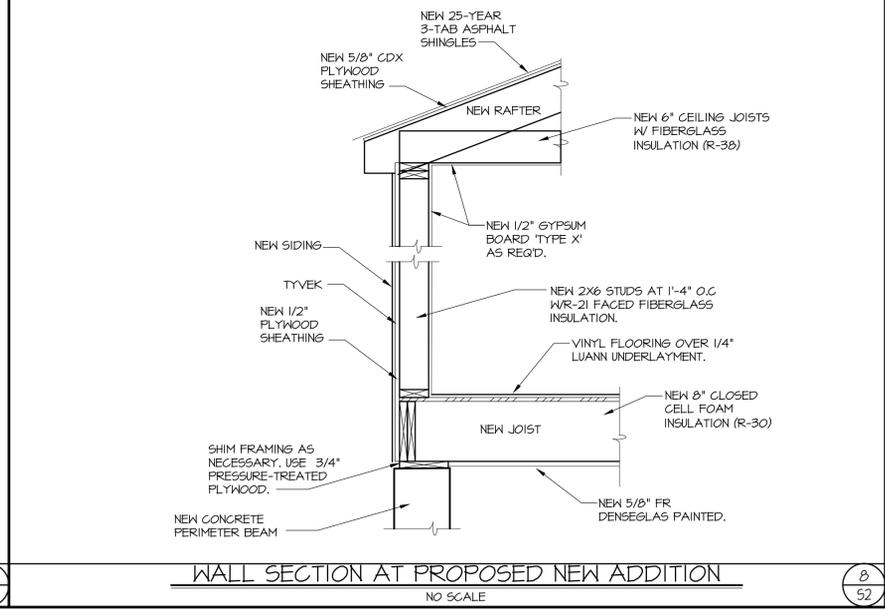
CONCRETE TO BLDG. CONNECTION DETAIL
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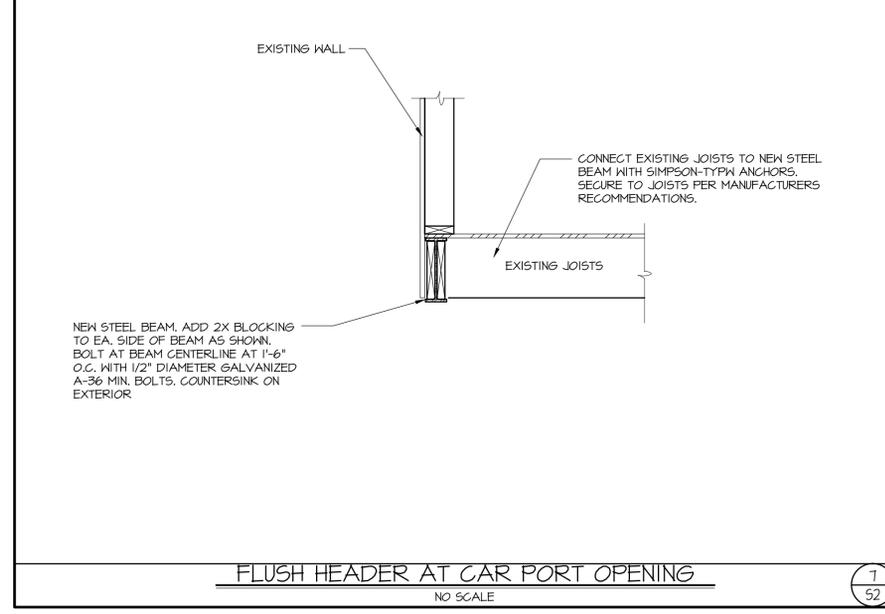
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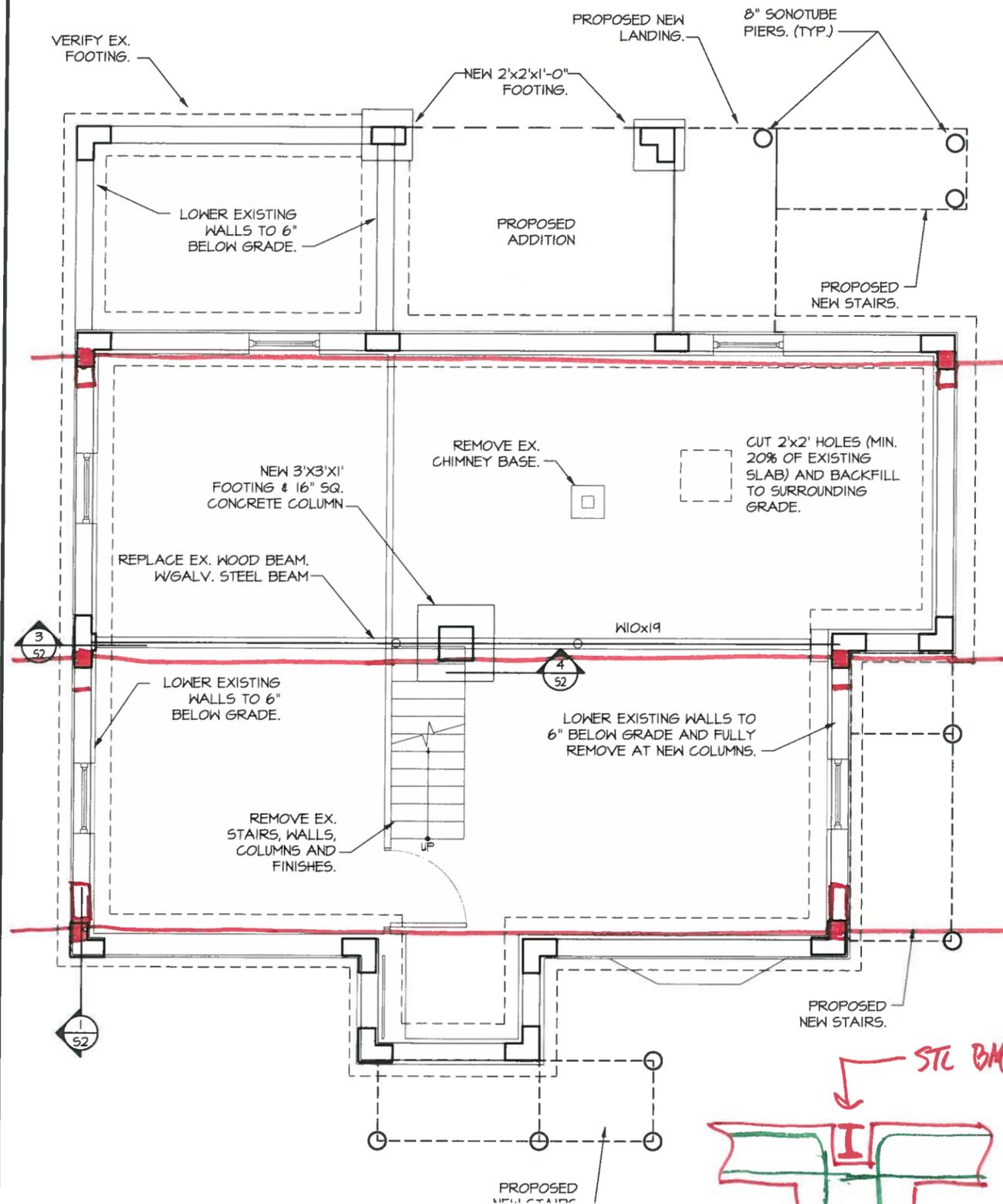
TYPICAL ROOF RAFTER STRAPPING DETAIL
NO SCALE



WALL SECTION AT PROPOSED NEW ADDITION
NO SCALE



FLUSH HEADER AT CAR PORT OPENING
NO SCALE



MAIN LIFTING STEEL

