

# **EROSION AND SEDIMENT CONTROL PLAN**

## **WIND COLEBROOK NORTH COLEBROOK, CONNECTICUT**

**Prepared for:**



**BNE Energy**  
29 South Main Street  
Town Center, Suite 200  
West Hartford, CT 06107

**by:**

**CIVIL 1**  
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**SEPTEMBER 2011**

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Appendix B	Inspection and Maintenance Records
Appendix C	Supporting Calculations

**Contact Information / Responsible Parties:**

Permittee(s):

BNE Energy  
29 South Main Street  
Town Center, Suite 200  
West Hartford, CT 06107  
(800) 450-0503

Contractor Co-Permittee:

To be determined

Contractor Operator(s):

To be determined

Stormwater Manager and SWPPP Contact(s):

BNE Energy  
29 South Main Street  
Town Center Suite 200  
West Hartford, CT 06107  
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Woodbury, CT 06798

**Section 1.0  
PROJECT INTRODUCTION**

## **1.0 PROJECT INTRODUCTION**

### **Project/Site Information:**

Project/Site Name: Wind Colebrook North

Location: Winsted- Norfolk Road  
Colebrook, Connecticut

Latitude/Longitude: Latitude: Longitude:  
41° 58' 30" N 73° 08' 28" W

Method for determining latitude/longitude: Google Earth

### **1.1 SITE SUMMARY**

#### ***1.1.1 Existing Conditions***

Located along the Winsted- Norfolk Road and Rock Hall Road, the project site consists of approximately 125 acres of primarily undeveloped property. Development on the property is limited to a seasonal use building in the southwest corner of the property which services a golf driving range. The Property is located approximately 1,050 feet east of the Norfolk town line town line and is located in Drainage Basin 4302 Mad River which is part of the larger Farmington Regional Basin as indentified in the Atlas of the Public Water Supply Sources and Drainage Basins of Connecticut . The surrounding land is predominately low density residential. The site will be accessed via Rock Hall Road. This access point will be maintained throughout the construction process. Currently, there are no structural stormwater discharge points. All stormwater flows over land to discharge points off site.

#### ***1.1.2 Project Description***

The Project consists of three GE 1.6MW wind turbines, associated ground equipment, the installation of an access driveway and an electrical connection. The installation of the turbines will require the construction of temporary equipment lay-down areas for the turbines, crane assembly area, access road and associated ground equipment including an electrical collector yard and associated utility infrastructure so that the turbines can be interconnected to the electrical grid. Following completion of the project, all temporary structures will be removed and the site returned to pre-construction conditions as far as practicable.

#### ***1.1.3 Site Specific Concerns***

The terrain and existing topography of the project site is such that during construction special care will be required to ensure that all BMPs remain intact and are functional.

#### ***1.1.4 Construction Schedule***

Currently specific dates for construction have not been determined but anticipate construction to begin in Fall 2011.

## **1.2 PROJECT OWNER AND OPERATOR**

The project owner and operator, BNE Energy, will be the responsible entity for completing the project. The address and telephone is:

BNE Energy  
29 South Main Street  
Town Center Suite 200  
West Hartford, CT 06107  
(800) 450-0503

## **1.3 SOILS, SLOPES, VEGETATION, AND CURRENT DRAINAGE PATTERNS**

### **1.3.1 Soil type(s)**

Based upon a review of typical geologic conditions and the National Soil Cooperative Survey, the soils have been classified as (1) Bice- Millsite complex 3-15% slopes, very rocky, (2) Bice- Millsite complex 15-45% slopes, very rocky, (3) Bice fine sandy loam 3-8% slopes, very stony, (4) Bice fine sandy loam 8-15% slopes, very stony, (5) Bice fine sandy loam 15-25% slopes, very stony, (6) Schroon fine sandy loam 2-15% slopes, very stony, (7) Shelburne fine sandy loam 3- 8% slopes, (8) Shelburne fine sandy loam 8-15% slopes, (9) Shelburne fine sandy loam 8-15% slopes, very stony and (10) Brayton- Loonmeadow complex extremely stony.

### **1.3.2 Slopes**

The project site consists of varying slope conditions ranging from relatively flat conditions in the central portion of the site and moderately steeper slopes in the extreme easterly and westerly portion of the site. The proposed towers are to be located on gentle slopes.

### **1.3.3 Drainage Patterns**

Existing site topography is such that runoff migrates, typically via overland sheet flow from the easterly and westerly upland portions of the site towards the wetlands in the center portion of the site. The flow through the wetlands is generally from the north to the south and southeast through the Mill Brook which is a perennial watercourse.

### **1.3.4 Vegetation**

Six major habitat types have been identified in the Terrestrial Wildlife Habitat & Wetland Impact Analysis prepared by VHB, Inc. These types include (1) second growth Northern Hardwood forest, (2) second growth Northern Hardwoods- Hemlock- White Pine forest, (3) early successional Northern Hardwood forest, (4) Palustrine forested wetlands (which include Mill Brook, a perennial watercourse), (5) Palustrine scrub- shrub- emergent wetlands and (6) maintained lawn (golf driving range).

## **1.4 SITE FEATURES AND SENSITIVE AREAS TO BE PROTECTED**

### **1.4.1 Receiving Waters and TMDL Applicability**

Mill Brook runs through the property and ultimately receives the storm water runoff from the site.. This water body is not considered impaired and is not listed on the most current 303(d) listing of impaired waterways.

**1.4.2 Wetlands**

Within to the property boundary a wetland has been identified and delineated. Mitigation and impacts are discussed in the environmental assessment completed by VHB, Inc.

**Section 2.0**  
**CONSTRUCTION ACTIVITIES**

## **2.0 CONSTRUCTION ACTIVITIES**

### **2.1 DESCRIPTION OF CONSTRUCTION ACTIVITY**

During construction, BNE will continuously monitor construction methodologies and procedures as well as site conditions to minimize adverse effects to the environment and public.

Construction will typically consist of activities such as:

- Surveys to stake access roads, structure locations and limits of clearing
- Land clearing
- Establishment of construction staging area
- Installation of sediment and erosion control devices
- Excavation and installation of access roads
- Excavation and installation of lay-down and equipment assembly areas
- Excavation and installation of foundations and erection of new structures
- Installation of conductors
- Restoration of site, including re-establishment of vegetative areas

### **2.2 CONSTRUCTION SITE ESTIMATES**

The following are estimates of the construction site:

Property Area: 125 acres

Area to be disturbed: 14.63 acres

Percentage impervious area before construction: 0%

Runoff coefficient number (RCN) before construction: 58

Percentage impervious area after construction: 0.98%

Runoff coefficient number (RCN) after construction: 60

Summary of peak flows: See Section 2.3.3 of the Stormwater Management Plan With Stormwater Pollution Prevention Plan Report

Summary of groundwater recharge: 0.015 AC-FT

**Section 3.0**  
**BEST MANAGEMENT PRACTICES**

### **3.0 BEST MANAGEMENT PRACTICES**

Soil erosion and sediment controls are measures that are used to reduce the amount of soil particles that are carried from a land area and deposited in receiving waters. This section provides a general description of the most appropriate control measures proposed for the Project. The permittee's construction contractor(s) and their subcontractors will be responsible for amending the erosion and sediment controls in the SWPPP for their portion(s) of the project as needed. Based on field conditions at the time of construction, the contractors or subcontractors may adjust the locations and types of BMPs so that erosion and sedimentation are controlled to the maximum extent practicable. However, in no case will modifications to the SWPPP result in any less stringent erosion and sedimentation control measures than specified herein.

#### **3.1 STRUCTURAL CONTROL PRACTICES**

Structural control practices divert flows from exposed soils, store water flow, or otherwise limit runoff from exposed areas of the site. Such practices may include silt fences, drainage swales, sediment traps, check dams, subsurface drains, pipe slope drains, rock outlet protection (rip-rap), reinforced soil retaining systems, and temporary or permanent sediment basins. Some of these practices may be used as both temporary and permanent control measures. Structural control practices should be placed in upland areas to the degree practicable to prevent erosion and reduce sedimentation in lower elevation areas. See Appendix A for additional information.

#### **3.2 TEMPORARY EROSION CONTROL PRACTICES**

Erosion and sediment control measures will be in place prior to the initiation of soil disturbing activities and will be maintained throughout construction. The contractor may need erosion control measures in other locations of the project as work progresses to keep sediment from leaving the construction site. These measures will be determined by the contractor in the field; if measures are changed in the field, the SWPPP must be modified accordingly. All temporary erosion controls will be removed after the protected area is finally stabilized. The minimum temporary erosion and sediment control practices that will be used for the Project are discussed in the following sections. See Appendix D for additional information.

##### **3.2.1 *Sediment Fence (GSF)***

Will retain sediment from small disturbed areas. Sediment fence will be placed along slopes as shown on construction details. The contractor will use his best judgment to install additional sediment fence as necessary to prevent loss of sediment. Refer to section 5-11 of 2002 Connecticut Guidelines for Soil Erosion and Sediment Control.

Maintenance: Inspect the silt fence at least once a week and within 24 hours of the end of a storm with a rainfall amount of 0.5 inches or greater to determine maintenance needs. When used for dewatering operations, inspect frequently before, during and after pumping operations. Remove the sediment deposits, or if room allows, install a second silt fence up slope from the existing fence when deposits reach approximately one half the height of the existing fence. Replace or repair within 24 hours of an observed failure. Refer to Connecticut Guidelines for Soil Erosion and Sediment Control figure GF-5 for troubleshooting failures. Maintain silt fence until the contributing area is stabilized.

### **3.2.2 Hay Bale Barrier (HB)**

Will retain sediment from small disturbed areas. Hay bales will be placed along slopes as shown on construction details. The contractor will use his best judgment to install additional hay bales as necessary to prevent loss of sediment. Refer to section 5-11 of 2002 Connecticut Guidelines for Soil and Sediment Control.

Maintenance: Inspect the hay bale barrier at least once a week and within 24 hours of the end of a storm with a rainfall amount of 0.5 inches or greater to determine maintenance needs. When used for dewatering operations, inspect frequently before, during and after pumping operations. Remove the sediment deposits, or if room allows, install a secondary barrier up slope from the existing barrier when deposits reach approximately one half the height of the barrier. Replace or repair within 24 hours of an observed failure. Refer to Connecticut Guidelines for Soil Erosion and Sediment Control figure HB-5 for troubleshooting failures. Maintain hay bale barrier until the contributing area is stabilized.

### **3.2.3 Stone Check Dam (SCD)**

Will be used to reduce velocity of concentrated flows, thus reducing erosion of the drainage way.

Maintenance: Inspect the stone check dam at least once a week and within 24 hours of the end of a storm with a rainfall amount of 0.5 inches or greater to determine maintenance needs. Remove the sediment deposits when deposits reach approximately one half the height of the Check dam. Replace or repair within 24 hours of an observed failure. Maintain until the contributing area is stabilized.

### **3.2.4 Permanent Diversion (PD)**

Will be a permanent component of the water management system used to protect downgradient areas from erosion and sedimentation. It will also direct water originating from undisturbed areas away from areas where construction activities are taking place.

Maintenance: When the permanent diversion is located within close proximity to on going construction activities, inspect the diversion at the end of each work day and immediately repair damage caused by construction equipment. Otherwise, inspect the permanent diversion and associated measures at least once a week and within 24 hours of the end of a storm with a rainfall amount of 0.5 inches or greater to determine maintenance needs. Repair within 24 hours of an observed failure.

### **3.2.5 Temporary Diversion (TD)**

Will be used to divert sediment laden runoff from a disturbed area to a sediment trapping facility. It will also direct water originating from undisturbed areas away from areas where construction activities are taking place.

Maintenance: When the temporary diversion is located within close proximity to on going construction activities, inspect the diversion at the end of each work day and immediately repair damage caused by construction equipment. Otherwise, inspect the temporary diversion and associated measures at least once a week and within 24 hours of the end of a storm with a rainfall

amount of 0.5 inches or greater to determine maintenance needs. Repair within 24 hours of an observed failure.

### **3.2.6 Temporary Sediment Trap (TST)**

Will be used to detain sediment laden runoff from small disturbed areas long enough to allow the majority of sediment to settle out.

Maintenance: Inspect the temporary sediment trap and associated controls at least once a week and within 24 hours of the end of a storm with a rainfall amount of 0.5 inches or greater to determine maintenance needs. Check the outlet to verify that it is structurally sound and has not been damaged by erosion or construction equipment. The height of the stone outlet should be maintained at least 1 foot below the crest of the embankment. When sediment has accumulated more than one quarter of the minimum wet storage volume, dewater and remove sediment as necessary to restore the trap to its original dimensions.

### **3.2.7 Construction Entrance (CE)**

Will be used to reduce tracking of sediment off site to paved areas.

Maintenance: Maintain the entrance in a condition which will prevent tracking and washing of sediment onto paved surfaces. Provide periodic top dressing with additional stone or additional length as required. Immediately remove all sediment spilled, dropped, washed or tracked onto paved surfaces.

### **3.2.8 Temporary Erosion Control Blankets (ECB)**

Will be used to provide temporary surface protection to disturbed soils to absorb raindrop impact and to reduce sheet and rill erosion until vegetation is established.

Maintenance: Inspect temporary erosion control blankets at least once a week and within 24 hours of the end of a storm with a rainfall amount of 0.5 inches or greater to determine maintenance needs. Repair any dislodged or failed blankets immediately.

## **3.3 SOIL STABILIZATION PRACTICES**

Soil stabilization involves covering disturbed soils with grass, mulch, straw, geotextiles, trees, vines, or shrubs. Stabilization practices for exposed disturbed soils are extremely important while conducting construction activities. Vegetative cover serves to reduce the erosion potential by absorbing the energy of raindrops, promoting infiltration in lieu of runoff, and reducing the velocity of runoff. Stabilization measures shall be initiated as soon as practicable, but no more than 14 days after construction activities have temporarily or permanently ceased on any portion of the site.

## **3.4 MAINTENANCE AND INSPECTIONS**

All erosion and sediment control devices shall be installed pursuant to the specifications in the construction details and in accordance with the Connecticut General Permit for the Discharge of Stormwater and Dewatering Wastewaters Associated with Construction Activities . They will be maintained so that they remain effective at all times.

Erosion and sediment control devices will be inspected by qualified personnel at least once every seven calendar days or within 24 hours of each 0.5-inch or greater rainfall event. During each inspection, the construction inspector will complete the Inspection and Maintenance Report Form located in the appendix. This form will be copied and used as necessary. Ineffective temporary erosion control measures will be repaired or replaced before the next storm event or as soon as practicable. The permittee will immediately install additional temporary erosion control devices in any area deemed in need of protection.

Following temporary or final stabilization, inspections must be conducted at least once a month. If construction has been halted due to frozen conditions, regular inspections are not mandatory until one month before the expected thaw. If vegetation establishment is not satisfactory, special steps to correct the problem will be implemented such as over seeding, mulching, sodding, or the use of erosion control blankets. Once a definable area of the construction site has been finally stabilized, no further inspection requirements apply to that area.

### **3.5 FINAL STABILIZATION**

#### **3.5.1 Seeding**

The contractor will be responsible for labor, materials, tools, equipment, and other related items required for preparing ground, providing for sowing of seeds, fertilizing, mulching and top dressing, and other management practices required for erosion control and to achieve final stabilization. It will be the contractor's responsibility to make sure that the soil seedbed is not blown, washed, or otherwise removed from the site. The contractor will make repairs (including replacement of lost topsoil and mulch) to the seedbed preparation site in the event of heavy rain, wind, or other natural events that cause damage. When practicable, native plant species should be used for landscaping.

#### **3.5.2 Fertilizer**

Soil in areas of disturbance may need supplementation from fertilizer. Soil tests may be necessary to determine the most appropriate fertilizer for each location. Once applied, the fertilizer will be worked into the soil to limit exposure to stormwater. Fertilizer spills will be cleaned up immediately and will not be applied along or in a waterway.

#### **3.5.3 Mulching**

Mulching will be used in conjunction with both temporary and permanent seeding practices to enhance success by providing erosion protection prior to the onset of vegetative growth. Mulches enhance plant establishment by moderating soil temperatures and conserving moisture. After seeding, straw or hay mulch will be applied at a rate of two to three tons per acre on the disturbed areas. Other forms of mulch will be applied at a rate designated by the Project Engineer. Mulch will not be applied in wetlands, on lawns, and areas where hydro-mulch is used. Mulch will be anchored immediately after placement on steep slopes and stream banks. Mulch will be held in place by a very thin covering of topsoil, small brush, pins, stakes, wire mesh, asphalt binder, or other adhesive material approved by the project engineer.

#### **3.5.4 Topsoiling**

Topsoil should be applied in areas where the subsoil or existing surface soil does not provide an adequate growth medium for the desired vegetation, where soil is too shallow to provide adequate

rooting depth, or where the soil contains substances toxic to the desired vegetation. Topsoil shall be reasonably free from subsoil and stumps, roots, brush, stones, and clay lumps or similar objects.

### ***3.5.5 Temporary Control Removal***

Temporary erosion controls will be left in place until the Project site is stabilized with a uniform vegetative cover of 70 percent density of vegetative cover on all unpaved areas. Following re-vegetation, the permittee will conduct periodic site visits to make sure that vegetation establishment is satisfactory. If sufficient vegetative cover has not been achieved, additional restoration measures will be implemented. Inspection results will be documented using the Inspection and Maintenance Report Form found in the appendix. All temporary soil erosion and sediment control measures will be removed and disposed of after final site stabilization is achieved and before submitting the Notice of Termination (NOT) to the CT DEP.

**Section 4.0**  
**EROSION CONTROL PLAN APPENDICES**

#### **4.0 SWPPP APPENDICES**

Attach the following documentation to the SWPPP in the following appendices.

##### Appendix A – Maps and Drawings

- Site Maps
- Site Plan

##### Appendix B – Inspection and Maintenance Records

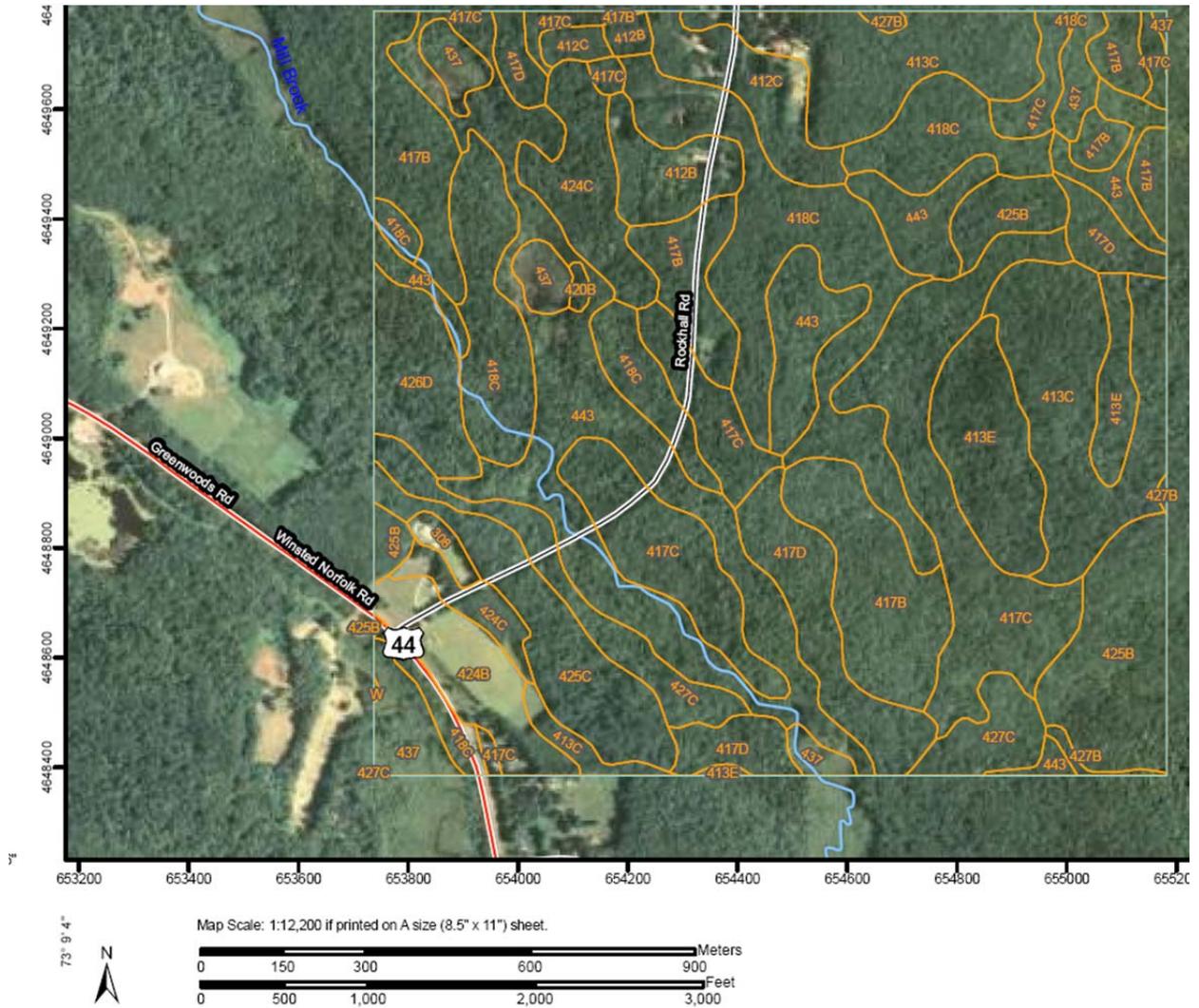
- Inspection & Maintenance Log
- Inspection Report
- Maintenance Report

##### Appendix C – Supporting Calculations

- Calculations for Swales, Culverts, Riprap and Temporary Sediment Traps

Appendix A  
**MAPS AND DRAWINGS**





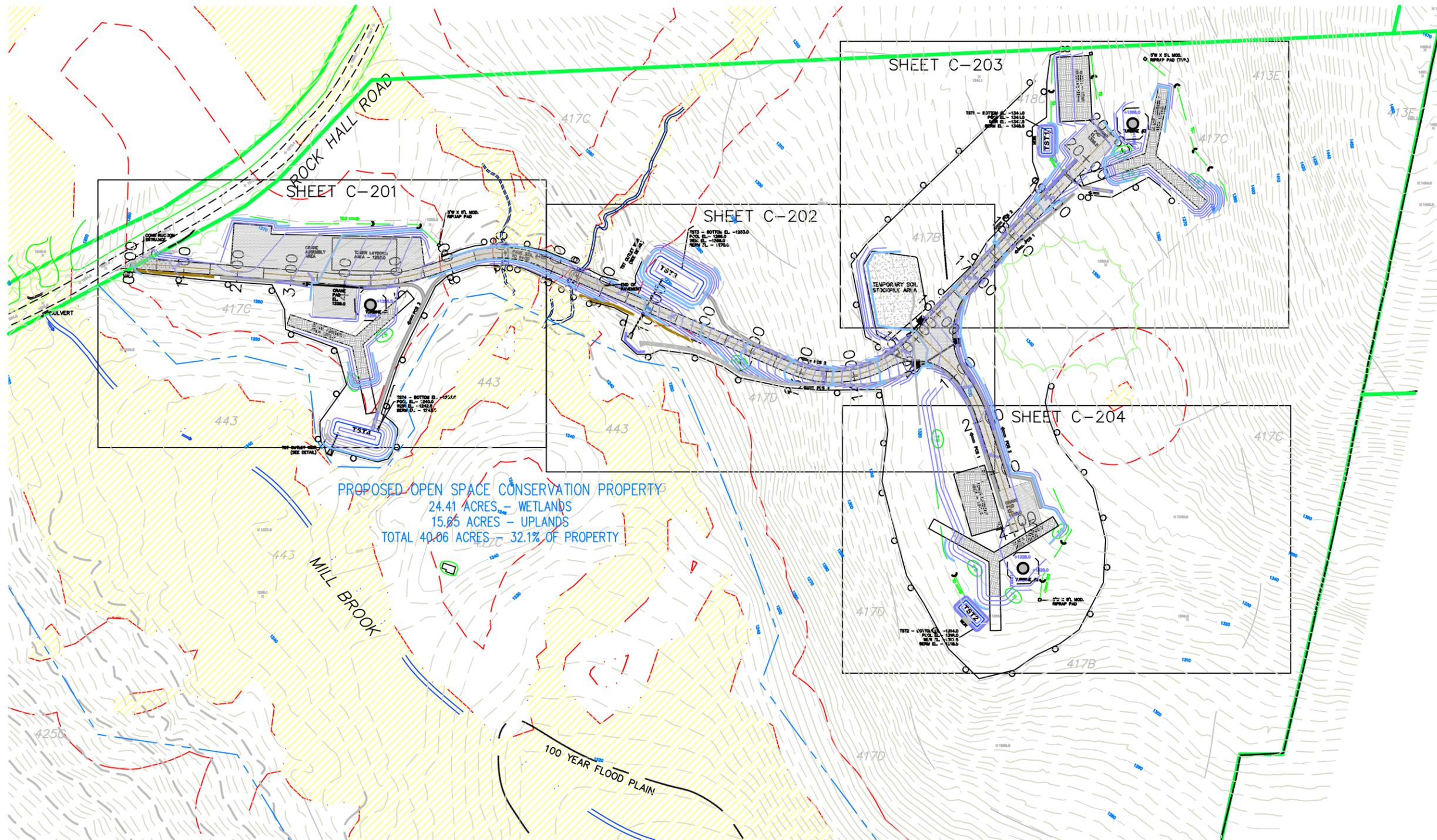
Soil Map--State of Connecticut

MAP LEGEND		MAP INFORMATION
<p><b>Area of Interest (AOI)</b></p> <p> Area of Interest (AOI)</p> <p><b>Soils</b></p> <p> Soil Map Units</p> <p><b>Special Point Features</b></p> <p> Blowout</p> <p> Borrow Pit</p> <p> Clay Spot</p> <p> Closed Depression</p> <p> Gravel Pit</p> <p> Gravelly Spot</p> <p> Landfill</p> <p> Lava Flow</p> <p> Marsh or swamp</p> <p> Mine or Quarry</p> <p> Miscellaneous Water</p> <p> Perennial Water</p> <p> Rock Outcrop</p> <p> Saline Spot</p> <p> Sandy Spot</p> <p> Severely Eroded Spot</p> <p> Sinkhole</p> <p> Slide or Slip</p> <p> Sodic Spot</p> <p> Spoil Area</p> <p> Stony Spot</p>	<p> Very Stony Spot</p> <p> Wet Spot</p> <p> Other</p> <p><b>Special Line Features</b></p> <p> Gully</p> <p> Short Steep Slope</p> <p> Other</p> <p><b>Political Features</b></p> <p> Cities</p> <p><b>Water Features</b></p> <p> Oceans</p> <p> Streams and Canals</p> <p><b>Transportation</b></p> <p> Rails</p> <p> Interstate Highways</p> <p> US Routes</p> <p> Major Roads</p> <p> Local Roads</p>	<p>Map Scale: 1:2,900 if printed on D size (22" x 34") sheet.</p> <p>The soil surveys that comprise your AOI were mapped at 1:12,000.</p> <p>Please rely on the bar scale on each map sheet for accurate map measurements.</p> <p>Source of Map: Natural Resources Conservation Service            Web Soil Survey URL: <a href="http://websoilsurvey.nrcs.usda.gov">http://websoilsurvey.nrcs.usda.gov</a>            Coordinate System: UTM Zone 18N NAD83</p> <p>This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.</p> <p>Soil Survey Area: State of Connecticut            Survey Area Data: Version 8, Dec 13, 2010</p> <p>Date(s) aerial images were photographed: 8/14/2006</p> <p>The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.</p>

Soil Map--State of Connecticut

## Map Unit Legend

State of Connecticut (CT600)			
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
61C	Canton and Charlton soils, 8 to 15 percent slopes, very stony	0.3	0.1%
62D	Canton and Charlton soils, 15 to 35 percent slopes, extremely stony	0.4	0.1%
412B	Bice fine sandy loam, 3 to 8 percent slopes	15.4	3.0%
412C	Bice fine sandy loam, 8 to 15 percent slopes	15.3	3.0%
413C	Bice-Millsite complex, 3 to 15 percent slopes, very rocky	64.9	12.8%
413E	Bice-Millsite complex, 15 to 45 percent slopes, very rocky	23.5	4.6%
417B	Bice fine sandy loam, 3 to 8 percent slopes, very stony	58.1	11.5%
417C	Bice fine sandy loam, 8 to 15 percent slopes, very stony	93.1	18.4%
417D	Bice fine sandy loam, 15 to 25 percent slopes, very stony	18.3	3.6%
418C	Schroon fine sandy loam, 2 to 15 percent slopes, very stony	57.5	11.3%
420B	Schroon fine sandy loam, 3 to 8 percent slopes	0.6	0.1%
424C	Shelburne fine sandy loam, 8 to 15 percent slopes	9.1	1.8%
425B	Shelburne fine sandy loam, 3 to 8 percent slopes, very stony	21.1	4.2%
425C	Shelburne fine sandy loam, 8 to 15 percent slopes, very stony	4.9	1.0%
426D	Shelburne fine sandy loam, 15 to 35 percent slopes, extremely stony	2.8	0.6%
427B	Ashfield fine sandy loam, 2 to 8 percent slopes, very stony	17.3	3.4%
427C	Ashfield fine sandy loam, 8 to 15 percent slopes, very stony	12.3	2.4%
428A	Ashfield fine sandy loam, 0 to 3 percent slopes	2.9	0.6%
437	Wonsqueak mucky peat	22.6	4.5%
443	Brayton-Loonmeadow complex, extremely stony	65.9	13.0%
<b>Totals for Area of Interest</b>		<b>506.5</b>	<b>100.0%</b>

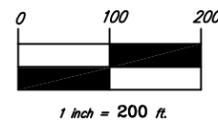


PROPOSED OPEN SPACE CONSERVATION PROPERTY  
 24.41 ACRES - WETLANDS  
 15.65 ACRES - UPLANDS  
 TOTAL 40.06 ACRES - 32.1% OF PROPERTY



NO.	REVISION	DATE

*Previous Editions Obsolete*



**BNE ENERGY, INC.**  
 29 SOUTH MAIN STREET  
 TOWN CENTER SUITE 200  
 WEST HARTFORD, CT 06107

**OVERALL  
 EROSION CONTROL PLAN**

**WIND COLEBROOK  
 NORTH**  
 ROCK HALL ROAD

COLEBROOK CONNECTICUT



CORNERSTONE PROFESSIONAL PARK, SUITE D-101  
 43 SHERMAN HILL ROAD  
 WOODBURY CONNECTICUT (203) 266-0778

SCALE: 1" = 200'  
 DATE: 23 SEP 11  
 PROJ. NO.: 3093  
 CADD FILE NAME: 3093  
 DRAWING NO.:

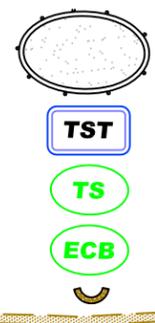
**C-200**

**LEGEND**

- PROPERTY LINE
- EXISTING CONTOUR
- PROPOSED CONTOUR
- PROPOSED STORM DRAINAGE
- EDGE OF WATER
- PROPOSED CONSERVATION PROPERTY
- WETLANDS/WATERCOURSE BOUNDARY
- 100' WETLANDS REVIEW AREA
- EXISTING ROADWAY
- PROPOSED GRAVEL ACCESS DRIVE

- COMPACTED EARTH
- LIMITS OF CLEARING
- TD → TEMP. WATER DIVERSION SWALE
- PCS → PERM. RIPRAP DIVERSION/CONVEYANCE SWALE
- SOIL TYPE BOUNDARY
- STAKED HAY BALES
- SILT FENCE/WOOD TURTLE EXCLUSIONARY FENCE
- BALED FILTER

- TEMP. SOIL STOCKPILE
- TEMPORARY SEDIMENT TRAP
- TEMPORARY SEEDING
- EROSION CONTROL BLANKET
- STONE CHECK DAM
- STONE INFILTRATION TRENCH































Appendix B  
**INSPECTION AND MAINTENANCE RECORDS**

**INSPECTOR CERTIFICATION**

Project:	Wind Colebrook North
Project Location:	Winsted- Norfolk Road Colebrook North
Contractor:	
Address:	
Phone:	
Fax:	

**CONSTRUCTION INSPECTION & MAINTENANCE LOG**

Date	Activity	Description	(1) Report No.
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	
	<input type="checkbox"/> Inspection <input type="checkbox"/> Maintenance	By: _____	

**CONSTRUCTION SITE INSPECTION REPORT**

General Information			
Project Name:	Wind Colebrook North		
Location:	Winsted- Norfolk Road Colebrook, Connecticut		
CT DEP Tracking No.		(1) Report No.	
Date of Inspection:		Start / End Time:	
Inspector's Name(s):			
Inspector's Title(s):			
Inspector's Contact Information:			
Describe present phase of construction:			
Type of Inspection: <input type="checkbox"/> Regular <input type="checkbox"/> Pre-storm event <input type="checkbox"/> During storm event <input type="checkbox"/> Post-storm event			
Weather Information			
Has it rained since the last inspection? <input type="checkbox"/> Yes <input type="checkbox"/> No			
If yes, provide: Storm Start Date & Time:                      Storm Duration (hrs):                      Approximate Rainfall (in):			
Weather at time of this inspection?			
Discharge Information (A)			
Do you suspect that discharges may have occurred since the last inspection? <input type="checkbox"/> Yes <input type="checkbox"/> No			
Are there any discharges at the time of inspection? <input type="checkbox"/> Yes <input type="checkbox"/> No			
Describe location of any discharges from the site:			

**SITE-SPECIFIC BMPs**

(B)	BMP Description	BMP Installed and Operating Properly?	Corrective Action Needed	Date for corrective action / responsible party
1		<input type="checkbox"/> Yes <input type="checkbox"/> No		
2		<input type="checkbox"/> Yes <input type="checkbox"/> No		
3		<input type="checkbox"/> Yes <input type="checkbox"/> No		
4		<input type="checkbox"/> Yes <input type="checkbox"/> No		
5		<input type="checkbox"/> Yes <input type="checkbox"/> No		
6		<input type="checkbox"/> Yes <input type="checkbox"/> No		
7		<input type="checkbox"/> Yes <input type="checkbox"/> No		
8		<input type="checkbox"/> Yes <input type="checkbox"/> No		
9		<input type="checkbox"/> Yes <input type="checkbox"/> No		
10		<input type="checkbox"/> Yes <input type="checkbox"/> No		
11		<input type="checkbox"/> Yes <input type="checkbox"/> No		
12		<input type="checkbox"/> Yes <input type="checkbox"/> No		
13		<input type="checkbox"/> Yes <input type="checkbox"/> No		
14		<input type="checkbox"/> Yes <input type="checkbox"/> No		
15		<input type="checkbox"/> Yes <input type="checkbox"/> No		
16		<input type="checkbox"/> Yes <input type="checkbox"/> No		
17		<input type="checkbox"/> Yes <input type="checkbox"/> No		
18		<input type="checkbox"/> Yes <input type="checkbox"/> No		
19		<input type="checkbox"/> Yes <input type="checkbox"/> No		

**OVERALL SITE ISSUES**

(C)	BMP/activity	Implemented?	Maintained?	Corrective Action	Date for corrective action/responsible person
1	Are all slopes and disturbed areas not actively being worked properly stabilized?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
2	Are natural resource areas (e.g., streams, wetlands, mature trees, etc.) protected with barriers or similar BMPs?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
3	Are perimeter controls and sediment barriers adequately installed (keyed into substrate) and maintained?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
4	Are discharge points and receiving waters free of sediment deposits?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
5	Are storm drain inlets properly protected?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
6	Is there evidence of sediment being tracked into the street?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
7	Is trash/litter from work areas collected and placed in covered dumpsters?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		

(C)	BMP/activity	Implemented?	Maintained?	Corrective Action	Date for corrective action/responsible person
8	Are washout facilities (e.g., paint, stucco, concrete) available, clearly marked, and maintained?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
9	Are vehicle and equipment fueling, cleaning, and maintenance areas free of spills, leaks, or any other deleterious material?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
10	Are materials that are potential stormwater contaminants stored inside or under cover?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
11	Are non-stormwater discharges (e.g., wash water, dewatering) properly controlled?	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
12	(Other)	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		
13	(Other)	<input type="checkbox"/> Yes <input type="checkbox"/> No	<input type="checkbox"/> Yes <input type="checkbox"/> No		

(C)	BMP/activity	Implemented?	Maintained?	Corrective Action	Date for corrective action/responsible person

**GENERAL INSPECTION COMMENTS AND EXPLANATION**

General Inspection Comments (D)
Is other descriptive information attached to this inspection report? <input type="checkbox"/> Yes <input type="checkbox"/> No

Plan Information (E)
Were all current plan BMP's in place at the time of inspection? <input type="checkbox"/> Yes <input type="checkbox"/> No
Are additional BMP's required? <input type="checkbox"/> Yes <input type="checkbox"/> No
Does the plan need to be updated? <input type="checkbox"/> Yes <input type="checkbox"/> No
Explanation of additional BMP and Plan update requirements:          

Certification statement:  
 I certify that I have thoroughly and completely reviewed the Stormwater Pollution Control Plan for the site. I further certify, based on such review and in my professional judgment, that the

Stormwater Pollution Control Plan has been prepared in accordance with the Connecticut Guidelines for Soil Erosion and Sediment Control, as amended, and the conditions for the General Permit for the Discharge of Stormwater and Dewatering Wastewaters from Construction Activities issued on October 1, 2002 (or as reissued or modified), and the controls required for such Plan are appropriate for the site. I am aware that there are significant penalties for false statements in this certification, including the possibility of fine and imprisonment for knowingly making false statements.

Name: \_\_\_\_\_

(Please print)

Signature: \_\_\_\_\_

Title: \_\_\_\_\_ Date: \_\_\_\_\_

.

**CONSTRUCTION SITE MAINTENANCE REPORT**

General Information			
Project Name:	Wind Colebrook North		
Location:	Winsted- Norfolk Road Colebrook, Connecticut		
CT DEP Tracking No.:		(1)	Report No.
Date of Maintenance:		Start / End Time:	
Describe present phase of construction:			
Type of Maintenance: <input type="checkbox"/> Regular <input type="checkbox"/> Pre-storm event <input type="checkbox"/> Post-storm event <input type="checkbox"/> Plan Update			
Maintenance Information			
Inspection Report Reference (No., Item)	Maintenance performed:		
Performed by:			
Inspection Report Reference (No., Item)	Maintenance performed:		
Performed by:			
Inspection Report Reference (No., Item)	Maintenance performed:		
Performed by:			
Inspection Report Reference (No., Item)	Maintenance performed:		
Performed by:			
Inspection Report Reference (No., Item)	Maintenance performed:		
Performed by:			

Inspection Report Reference (No., Item)	Maintenance performed:
Performed by:	
Inspection Report Reference (No., Item)	Maintenance performed:
Performed by:	
Inspection Report Reference (No., Item)	Maintenance performed:
Performed by:	
Inspection Report Reference (No., Item)	Maintenance performed:
Performed by:	
Inspection Report Reference (No., Item)	Maintenance performed:
Performed by:	
Inspection Report Reference (No., Item)	Maintenance performed:
Performed by:	
Inspection Report Reference (No., Item)	Maintenance performed:
Performed by:	

**Certification statement:**

I certify that I have thoroughly and completely reviewed the Stormwater Pollution Control Plan for the site. I further certify, based on such review and in my professional judgment, that the Stormwater Pollution Control Plan has been prepared in accordance with the Connecticut Guidelines for Soil Erosion and Sediment Control, as amended, and the conditions for the General Permit for the Discharge of Stormwater and Dewatering Wastewaters from Construction Activities issued on October 1, 2002 (or as reissued or modified), and the controls required for such Plan are appropriate for the site. I am aware that there are significant penalties for false statements in this certification, including the possibility of fine and imprisonment for knowingly making false statements.

Name: \_\_\_\_\_

Signature: \_\_\_\_\_

Title: \_\_\_\_\_ Date: \_\_\_\_\_

Appendix C  
**SUPPORTING CALCULATIONS**

**CALCULATIONS FOR SWALES, CULVERTS, RIP RAP  
AND TEMPORARY SEDIMENT TRAPS**

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

DRAINAGE CALCULATIONS FOR TEMPORARY  
DIVERSION #1 (TD1)

10-YEAR DESIGN STORM  
**9-23-11**

**Rational Method:**

$$Q = CIA$$

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 2.23 ac.

Proposed Land Cover

Grass = 0.15 ac.

Wooded = 2.08 ac.

$$C = [(.3 \cdot .15) + (.2 \cdot 2.08)] / 2.23 = 0.21$$

Time of Concentration = 20 minutes  $\therefore I = 3.6$  in/hr

$$Q = C \cdot I \cdot A = .21 \cdot 3.6 \cdot 2.23 = 1.69 \text{ cfs}$$

Velocity in grass-lined swale at 5% slope = 3.4 fps.

## Worksheet Worksheet for Trapezoidal Channel

Project Description	
Worksheet	TD1
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data	
Mannings Coeffic	0.030
Channel Slope	050000 ft/ft
Left Side Slope	2.00 H : V
Right Side Slope	2.00 H : V
Bottom Width	2.00 ft
Discharge	1.69 cfs

Results	
Depth	0.21 ft
Flow Area	0.5 ft <sup>2</sup>
Wetted Perim	2.92 ft
Top Width	2.82 ft
Critical Depth	0.26 ft
Critical Slope	0.023191 ft/ft
Velocity	3.40 ft/s ← GRASS LINED SWALE
Velocity Head	0.18 ft
Specific Energ	0.39 ft
Froude Numb	1.43
Flow Type	Supercritical

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

DRAINAGE CALCULATIONS FOR TEMPORARY  
DIVERSION #2 (TD2)

10-YEAR DESIGN STORM  
**9-23-11**

**Rational Method:**

Q = CIA

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 0.73 ac.

Proposed Land Cover

Grass = 0.05 ac.

Wooded = 0.68 ac.

$$C = [(.3*.05)+(.2*0.68)]/.73 = 0.21$$

Time of Concentration = 12 minutes ∴ I = 4.5 in/hr

$$Q = C*I*A = .21 * 4.5 * 0.73 = 0.70 \text{ cfs}$$

Velocity in grass-lined swale at max 14% slope = 3.51 fps.

# Worksheet

## Worksheet for Trapezoidal Channel

---

Project Description	
Worksheet	TD2
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

---

---

Input Data	
Mannings Coeffc	0.030
Channel Slope	140000 ft/ft
Left Side Slope	2.00 H : V
Right Side Slope	2.00 H : V
Bottom Width	2.00 ft
Discharge	0.70 cfs

---

---

Results	
Depth	0.09 ft
Flow Area	0.2 ft <sup>2</sup>
Wetted Perim	2.41 ft
Top Width	2.37 ft
Critical Depth	0.15 ft
Critical Slope	0.026813 ft/ft
Velocity	3.51 ft/s ← GRASS-LINED SWALE
Velocity Head	0.19 ft
Specific Energ	0.28 ft
Froude Numb	2.13
Flow Type	supercritical

---

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

DRAINAGE CALCULATIONS FOR TEMPORARY  
DIVERSION #3 (TD3)

10-YEAR DESIGN STORM  
**9-23-11**

**Rational Method:**

Q = CIA

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 0.62 ac.

Proposed Land Cover

Grass = 0.06 ac.

Wooded = 0.56 ac.

$$C = [(.3 \cdot .06) + (.2 \cdot 0.56)] / 0.62 = 0.21$$

Time of Concentration = 25 minutes  $\therefore I = 3.2$  in/hr

$$Q = C \cdot I \cdot A = .21 \cdot 3.2 \cdot 0.62 = 0.42 \text{ cfs}$$

Velocity in grass-lined swale at 12% slope = 2.78 fps.

# Worksheet

## Worksheet for Trapezoidal Channel

---

Project Description	
Worksheet	TD3
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

---

---

Input Data	
Mannings Coeffic	0.030
Channel Slope	120000 ft/ft
Left Side Slope	2.00 H : V
Right Side Slope	2.00 H : V
Bottom Width	2.00 ft
Discharge	0.42 cfs

---

---

Results	
Depth	0.07 ft
Flow Area	0.2 ft <sup>2</sup>
Wetted Perim	2.32 ft
Top Width	2.28 ft
Critical Depth	0.11 ft
Critical Slope	0.029456 ft/ft
Velocity	2.78 ft/s ← GRASS LINED SWALE
Velocity Head	0.12 ft
Specific Energ	0.19 ft
Froude Numb	1.91
Flow Type	supercritical

---

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

DRAINAGE CALCULATIONS FOR PERMANENT  
DIVERSION SWALE #1(PDS1)

25-YEAR DESIGN STORM  
**9-23-11**

**Rational Method:**

Q = CIA

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 2.91 ac.

Proposed Land Cover

Grass = 0.25 ac.

Wooded = 2.66 ac.

$C = [(.3*.25)+(.2*2.66)]/2.91 = 0.21$

Time of Concentration = 18 minutes ∴ I = 4.4 in/hr

$Q = C*I*A = .21 * 4.4 * 2.91 = 2.69$  cfs

Velocity in riprap-lined swale at 9.0% slope = 3.97 fps.

# Worksheet

## Worksheet for Trapezoidal Channel

---

Project Description	
Worksheet	PDS1
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

---

---

Input Data	
Mannings Coeffic	0.040
Channel Slope	090000 ft/ft
Left Side Slope	2.00 H : V
Right Side Slope	2.00 H : V
Bottom Width	2.00 ft
Discharge	2.69 cfs

---

---

Results	
Depth	0.27 ft
Flow Area	0.7 ft <sup>2</sup>
Wetted Perim	3.20 ft
Top Width	3.07 ft
Critical Depth	0.34 ft
Critical Slope	0.038339 ft/ft
Velocity	3.97 ft/s ← MOD RIPRAP SWALE
Velocity Head	0.24 ft
Specific Energ	0.51 ft
Froude Numb.	1.49
Flow Type	supercritical

---

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

DRAINAGE CALCULATIONS FOR PERMANENT  
DIVERSION SWALE #2(PDS2)

25-YEAR DESIGN STORM  
**9-23-11**

**Rational Method:**

$Q = CIA$

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 5.73 ac.

Proposed Land Cover

Grass = .10 ac.

Wooded = 5.63 ac.

$C = [(.3 \cdot .10) + (.2 \cdot 5.63)] / 5.73 = 0.20$

Time of Concentration = 21 minutes  $\therefore I = 4.1$  in/hr

$Q = C \cdot I \cdot A = .20 \cdot 4.1 \cdot 5.73 = 4.70$  cfs

Velocity in riprap-lined swale at 4.5% slope = 3.69 fps.

# Worksheet

## Worksheet for Trapezoidal Channel

---

Project Description	
Worksheet	PDS2
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

---

---

Input Data	
Mannings Coeffic	0.040
Channel Slope	045000 ft/ft
Left Side Slope	2.00 H : V
Right Side Slope	2.00 H : V
Bottom Width	2.00 ft
Discharge	4.70 cfs

---

---

Results	
Depth	0.44 ft
Flow Area	1.3 ft <sup>2</sup>
Wetted Perim	3.97 ft
Top Width	3.77 ft
Critical Depth	0.47 ft
Critical Slope	0.035378 ft/ft
Velocity	3.69 ft/s ← MOD. RIPRAP SWALE
Velocity Head	0.21 ft
Specific Energ	0.65 ft
Froude Numb	1.12
Flow Type	supercritical

---

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

DRAINAGE CALCULATIONS FOR CROSS  
CULVERT TO LEVEL SPREADER  
TURBINE 2 ACCESS ROAD STATION 0+60L

25-YEAR DESIGN STORM  
**9-23-11**

**Rational Method:**

$Q = CIA$

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 8.64 ac.

Proposed Land Cover

Grass = 0.35 ac.

Wooded = 8.29 ac.

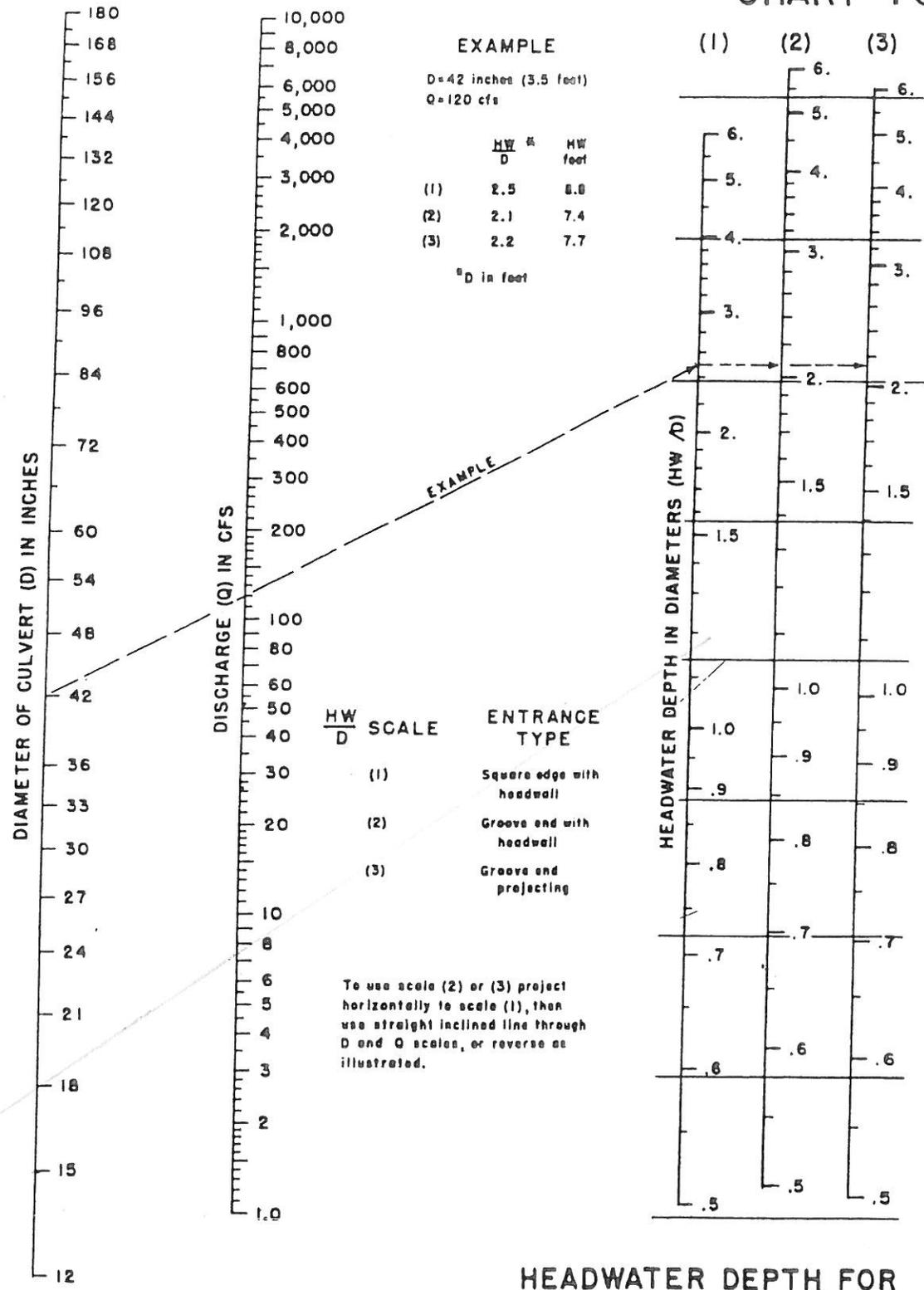
$C = [(.3 * .35) + (.2 * 8.29)] / 8.64 = 0.20$

Time of Concentration = 21 minutes  $\therefore I = 4.1$  in/hr

$Q = C * I * A = .20 * 4.1 * 8.64 = 7.08$  cfs

For 18" culvert - HW/D = 1.07

# CHART 1



## HEADWATER DEPTH FOR CONCRETE PIPE CULVERTS WITH INLET CONTROL

HEADWATER SCALES 2B3  
 REVISED MAY 1964

BUREAU OF PUBLIC ROADS JAN. 1963

CROSS CULVERT TURBINE 2  
 ACCESS ROAD STA. 0+60L

OUTLET PROTECTION - OUTLET VELOCITY ≤ 14 feet/sec

DISCHARGE (cfs)	OUTLET PIPE DIAMETER OR SPAN (in)										
	12	15	18	24	30	36	42	48	54	60	
0-5	<b>10</b>	<b>10</b>		<i>USE</i>							
6	12	11									
7		13	<b>12</b>								
8		14	13	<b>12</b>		<i>MINIMUM</i>					
9			14	13							
10			15	13							
11			16	14				<i>LENGTH</i>			
12				14							
14				16	<b>14</b>						
16				17	15	<b>14</b>				<i>OUTLINED</i>	
18				18	16	15					
20					17	15	<b>14</b>				
22		<i>USE</i>			18	16	15				
24						17	15	<b>14</b>			
26						17	16	15			
28						18	16	15			
30						19	17	16			
35						20	18	17	<b>16</b>		
40			<i>PREFORMED</i>					20	18	17	<b>16</b>
45							21	19	18	16	
50							22	20	18	17	
55								21	19	18	
60								22	20	19	
65								24	21	20	
70						<i>SCOUR</i>		25	22	20	
75								26	23	21	
80									24	22	
90									26	24	
100									28	25	
110										27	
125							<i>HOLE</i>			29	
130										30	

Table 8-6.1 - Length - L<sub>a</sub> (feet)  
Type A Riprap Apron

- Notes: 1. Bold face outlined boxes indicate minimum L<sub>a</sub> to be used for a given pipe diameter or span.
- 2. Rounding and interpolating are acceptable.

$$W = 3(L_{sp}) + 0.4(L_a) = 9.3', \text{ use } 10'$$

USE 10' W x 12' L MOD. RIPRAP APRON

**Cross culvert - Turbine 2 Access Road  
Worksheet for Circular Channel**

STA. 0+60L

Project Description	
Worksheet	Circular Channel
Flow Element	Circular Channel
Method	Manning's Formu
Solve For	Channel Depth

Input Data	
Mannings Coeffic	0.013
Channel Slope	010000 ft/ft
Diameter	18.0 in
Discharge	7.01 cfs

Results	
Depth	0.90 ft
Flow Area	1.1 ft <sup>2</sup>
Wetted Perime	2.65 ft
Top Width	0.00 ft
Critical Depth	1.03 ft
Percent Full	59.7 %
Critical Slope	0.006774 ft/ft
Velocity	6.37 ft/s ← USE MOD. RIPRAP
Velocity Head	0.63 ft
Specific Energ	1.53 ft
Froude Numbe	1.30
Maximum Disc	11.30 cfs
Discharge Full	10.50 cfs
Slope Full	0.004454 ft/ft
Flow Type	supercritical

*Wind Colebrook North  
Winsted-Norfolk Road*

DRAINAGE CALCULATIONS FOR PERMANENT  
DIVERSION SWALE #3(PDS3)

25-YEAR DESIGN STORM  
**9-23-11**

**Rational Method:**

$$Q = CIA$$

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 8.86 ac.

Proposed Land Cover

Grass = 0.70 ac.

Wooded = 8.16 ac.

$$C = [(.3*.70)+(.2*8.16)]/8.86= 0.21$$

Time of Concentration = 22 minutes  $\therefore$  I = 4.0 in/hr

$$Q = C*I*A = .21 * 4.0 * 8.86 = 7.44 \text{ cfs}$$

Velocity in riprap-lined swale at max 17.0% slope = 6.75 fps.

# Worksheet

## Worksheet for Trapezoidal Channel

---

Project Description	
Worksheet	PDS3
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

---

---

Input Data	
Mannings Coeffic	0.040
Channel Slope	170000 ft/ft
Left Side Slope	2.00 H : V
Right Side Slope	2.00 H : V
Bottom Width	2.00 ft
Discharge	7.44 cfs

---

---

Results	
Depth	0.40 ft
Flow Area	1.1 ft <sup>2</sup>
Wetted Perim	3.77 ft
Top Width	3.58 ft
Critical Depth	0.61 ft
Critical Slope	0.033251 ft/ft
Velocity	6.75 ft/s ← MOD. RIPRAP SWALE
Velocity Head	0.71 ft
Specific Energ	1.10 ft
Froude Numb	2.14
Flow Type	supercritical

---

## Worksheet Worksheet for Circular Channel

STORMWATER POND 1 OUTLET

Project Description	
Worksheet	Pond 1 outlet
Flow Element	Circular Chann
Method	Manning's Forr
Solve For	Channel Depth

Input Data	
Mannings Coeffic	0.013
Channel Slope	050000 ft/ft
Diameter	15.0 in
Discharge	2.65 cfs

Results	
Depth	0.36 ft
Flow Area	0.3 ft <sup>2</sup>
Wetted Perime	1.42 ft
Top Width	0.00 ft
Critical Depth	0.65 ft
Percent Full	29.0 %
Critical Slope	0.005816 ft/ft
Velocity	8.97 ft/s ← USE INT. RIPRAP PAD
Velocity Head	1.25 ft
Specific Energ	1.61 ft
Froude Numbe	3.10
Maximum Disc	15.54 cfs
Discharge Full	14.44 cfs
Slope Full	0.001683 ft/ft
Flow Type	supercritical

OUTLET PROTECTION - OUTLET VELOCITY ≤ 14 feet/sec

DISCHARGE (cfs)	OUTLET PIPE DIAMETER OR SPAN (in)										
	12	15	18	24	30	36	42	48	54	60	
0-5	10	10		USE							
6	12	11									
7		13	12								
8		14	13	12		MINIMUM					
9			14	13							
10			15	13							
11			16	14				LENGTH			
12				14							
14				16	14						
16				17	15	14			OUTLINED		
18				18	16	15					
20					17	15	14				
22		USE			18	16	15				
24						17	15	14			
26						17	16	15			
28						18	16	15			
30						19	17	16			
35						20	18	17	16		
40		PREFORMED						20	18	17	16
45							21	19	18	16	
50							22	20	18	17	
55								21	19	18	
60								22	20	19	
65								24	21	20	
70					SCOUR			25	22	20	
75								26	23	21	
80									24	22	
90									26	24	
100									28	25	
110										27	
125						HOLE				29	
130										30	

Table 8-6.1 - Length - L<sub>a</sub> (feet)  
Type A Riprap Apron

- Notes: 1. Bold face outlined boxes indicate minimum L<sub>a</sub> to be used for a given pipe diameter or span.
- 2. Rounding and interpolating are acceptable.

$$W = 3(S_p) + 0.4 L_a = 7.75', \text{ USE } 8'$$

8' W x 10' L INT. RIPRAP PAD

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

**DRAINAGE CALCULATIONS FOR CROSS CULVERT  
UNDER MAIN ACCESS DRIVE STATION 19+53**

**25-YEAR DESIGN STORM  
9-23-11**

**Rational Method:**

Q = CIA

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 1.24 ac.

Proposed Land Cover

Impervious = 0.05

Grass = 0.33 ac.

Wooded = 0.86 ac.

$C = [(.9 \cdot .05) + (.3 \cdot .33) + (.2 \cdot .86)] / 1.24 = 0.24$

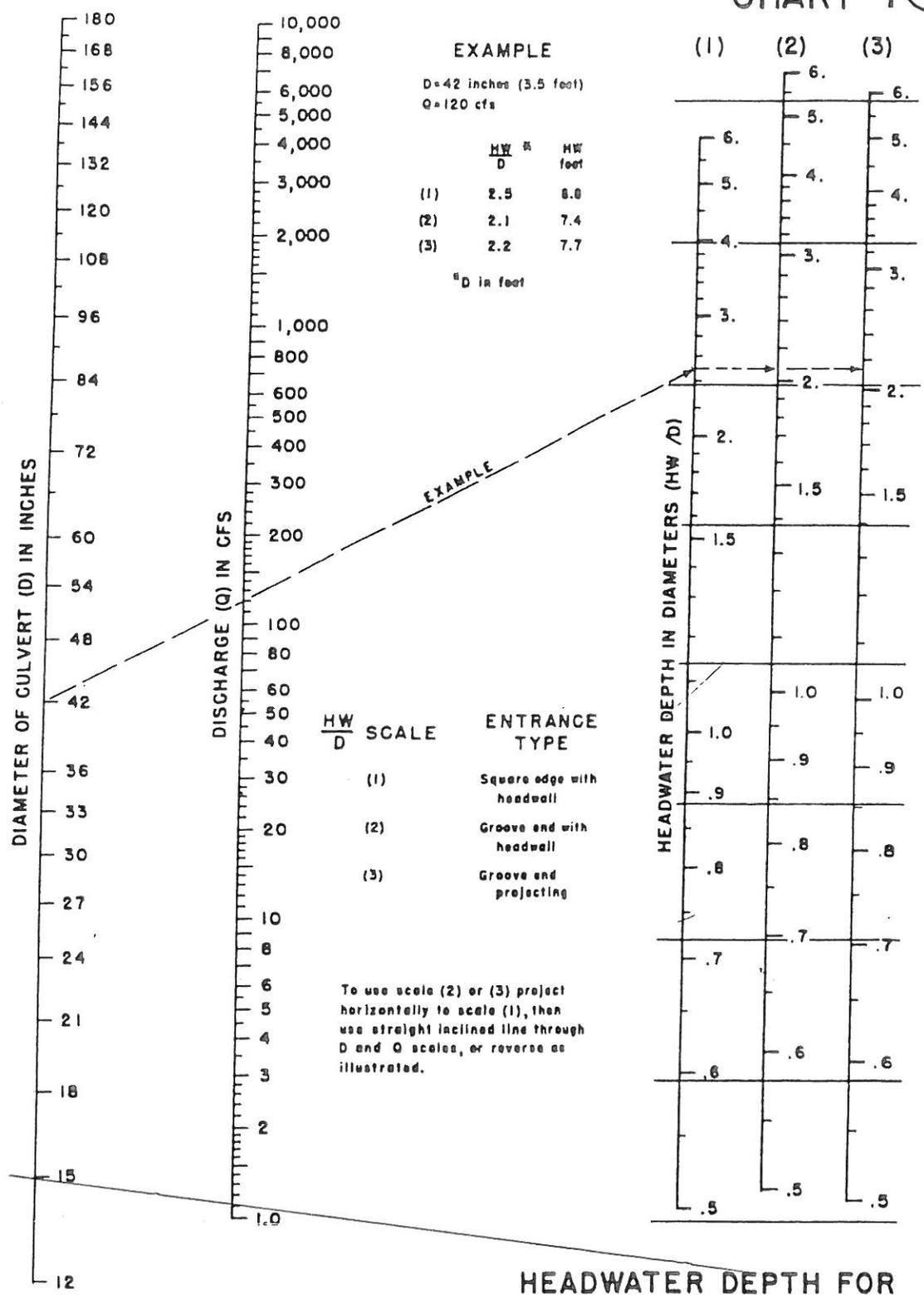
Time of Concentration = 20 minutes  $\therefore I = 3.6$  in/hr

$Q = C \cdot I \cdot A = .24 \cdot 3.6 \cdot 1.24 = 1.14$  cfs

For 15" culvert - HW/D = <0.5

Use 5' X 5' modified riprap pad at outlet into swale

# CHART 1



## HEADWATER DEPTH FOR CONCRETE PIPE CULVERTS WITH INLET CONTROL

HEADWATER SCALES 283  
REVISED MAY 1964

BUREAU OF PUBLIC ROADS JAN. 1963

CROSS CULVERT MAIN ACCESS  
DRIVE STA. 19+53

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

**DRAINAGE CALCULATIONS FOR PERMANENT  
CONVEYANCE SWALE #1 (PCS1) AND CROSS CULVERT  
UNDER MAIN ACCESS DRIVE AT STATION 15+10**

**25-YEAR DESIGN STORM  
9-23-11**

**Rational Method:**

Q = CIA

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 0.35 ac.

Proposed Land Cover

Grass = 0.07 ac.

Impervious = 0.28 ac.

$C = [(.3 \cdot .07) + (.9 \cdot .28)] / .35 = 0.79$

Time of Concentration = 5 minutes  $\therefore I = 6.7$  in/hr

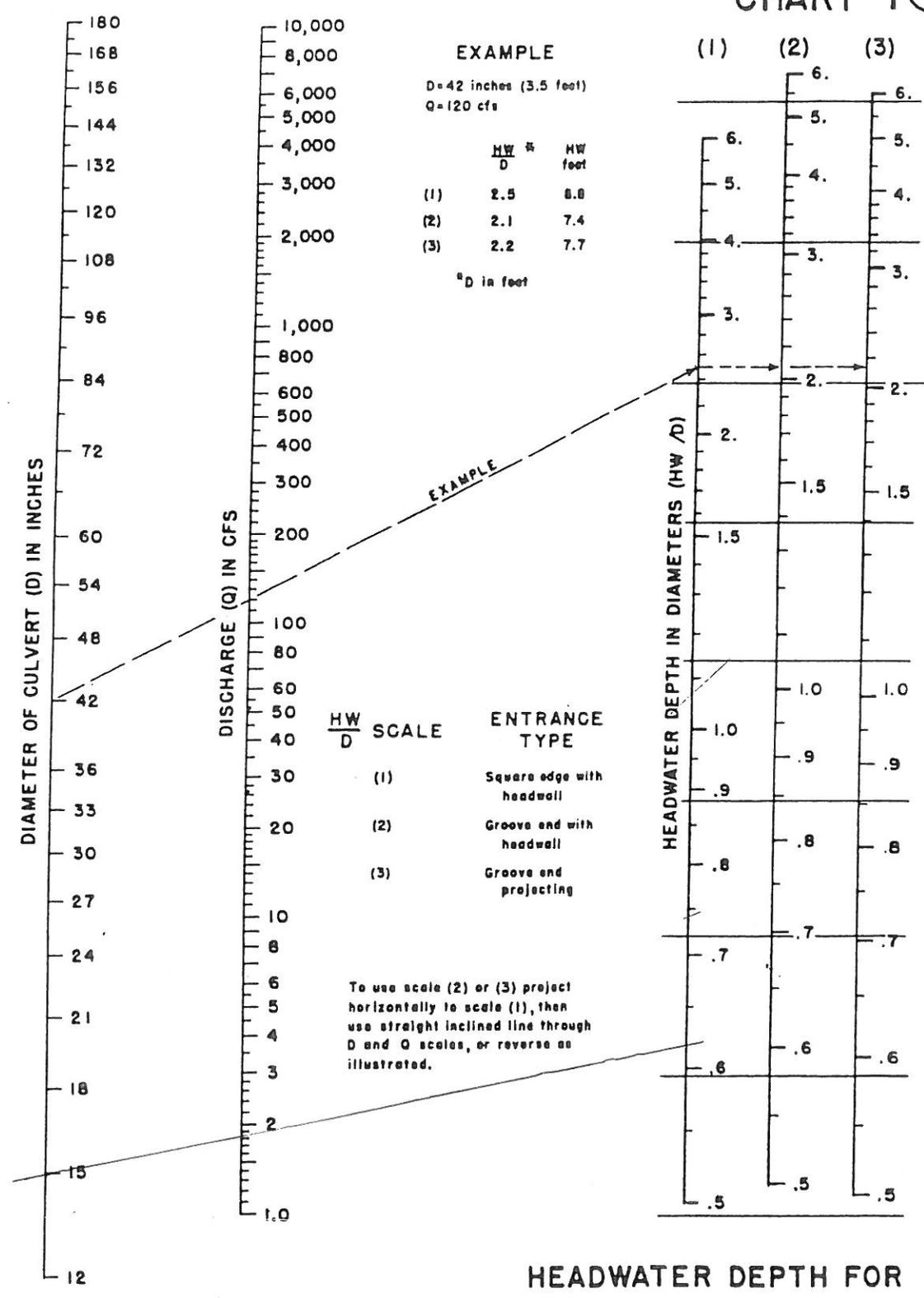
$Q = C \cdot I \cdot A = .79 \cdot 6.7 \cdot 0.35 = 1.86$  cfs

Velocity in riprap swale at max 8% slope = 3.39 fps

For 15" culvert - HW/D = 0.65

Use 5' X 5' modified riprap pad at outlet into swale

# CHART 1



## HEADWATER DEPTH FOR CONCRETE PIPE CULVERTS WITH INLET CONTROL

HEADWATER SCALES 283  
 REVISED MAY 1964

BUREAU OF PUBLIC ROADS JAN. 1963

CROSS CULVERT  
 MAIN ACCESS DRIVE  
 STA. 15+10

# Worksheet

## Worksheet for Trapezoidal Channel

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Project Description	
Worksheet	PCS1
Flow Element	Trapezoidal Cha
Method	Manning's Formi
Solve For	Channel Depth

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Input Data	
Mannings Coeffic	0.040
Channel Slope	080000 ft/ft
Left Side Slope	2.00 H : V
Right Side Slope	2.00 H : V
Bottom Width	2.00 ft
Discharge	1.86 cfs

---

---

Results	
Depth	0.22 ft
Flow Area	0.5 ft <sup>2</sup>
Wetted Perim	3.00 ft
Top Width	2.90 ft
Critical Depth	0.27 ft
Critical Slope	0.040520 ft/ft
Velocity	3.39 ft/s ← MOD. RIPRAP SWALE
Velocity Head	0.18 ft
Specific Enerç	0.40 ft
Froude Numb	1.37
Flow Type	supercritical

---

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

DRAINAGE CALCULATIONS FOR PERMANENT  
CONVEYANCE SWALE #2 (PCS2)

25-YEAR DESIGN STORM  
**9-23-11**

**Rational Method:**

$Q = CIA$

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 4.65 ac.

Proposed Land Cover

Impervious = 0.75 ac.

Grass = 0.40 ac.

Wooded = 3.50 ac.

$C = [(.9*.75)+(.3*.40)+(.2*3.50)]/4.65 = 0.32$

Time of Concentration = 21 minutes  $\therefore I = 4.1$  in/hr

$Q = C*I*A = .32 * 4.1 * 4.65 = 6.10$  cfs

Velocity in riprap-lined swale at max 15% slope = 6.09 fps

# Worksheet

## Worksheet for Trapezoidal Channel

---

Project Description	
Worksheet	PCS2
Flow Element	Trapezoidal Cha
Method	Manning's Formi
Solve For	Channel Depth

---

---

Input Data	
Mannings Coeffic	0.040
Channel Slope	150000 ft/ft
Left Side Slope	2.00 H : V
Right Side Slope	2.00 H : V
Bottom Width	2.00 ft
Discharge	6.10 cfs

---

---

Results	
Depth	0.37 ft
Flow Area	1.0 ft <sup>2</sup>
Wetted Perim	3.64 ft
Top Width	3.47 ft
Critical Depth	0.55 ft
Critical Slope	0.034142 ft/ft
Velocity	6.09 ft/s ← MOD. RIPRAP SWALE
Velocity Head	0.58 ft
Specific Enerç	0.94 ft
Froude Numb	2.00
Flow Type	supercritical

---

*Wind Colebrook North  
Winsted-Norfolk Road  
Colebrook, CT*

DRAINAGE CALCULATIONS FOR PERMANENT  
CONVEYANCE SWALE #3 (PCS3)

25-YEAR DESIGN STORM  
**9-23-11**

**Rational Method:**

$Q = CIA$

Where:

Q = flow rate (cfs)

C = runoff coefficient

I = rainfall intensity (in/hr)

A = area (ac.)

Total area contributing to swale: 1.70 ac.

Proposed Land Cover

Impervious = 0.25 ac.

Grass = 0.85 ac.

Wooded = 0.60 ac.

$$C = [(.9*.25)+(.3*0.85)+(.2*.60)]/1.70 = 0.60$$

Time of Concentration = 25 minutes  $\therefore I = 3.2$  in/hr

$$Q = C*I*A = .60 * 3.2 * 1.70 = 3.26 \text{ cfs}$$

Velocity in riprap-lined swale at max 8.5% slope = 4.13 fps

## Worksheet Worksheet for Trapezoidal Channel

Project Description	
Worksheet	PCS3
Flow Element	Trapezoidal Cha
Method	Manning's Formi
Solve For	Channel Depth

Input Data	
Mannings Coeffic	0.040
Channel Slope	085000 ft/ft
Left Side Slope	2.00 H : V
Right Side Slope	2.00 H : V
Bottom Width	2.00 ft
Discharge	3.26 cfs

Results	
Depth	0.30 ft
Flow Area	0.8 ft <sup>2</sup>
Wetted Perim	3.36 ft
Top Width	3.21 ft
Critical Depth	0.38 ft
Critical Slope	0.037280 ft/ft
Velocity	4.13 ft/s ← MOD. RIPRAP SWALE
Velocity Head	0.26 ft
Specific Energ	0.57 ft
Froude Numb	1.47
Flow Type	supercritical

STORMWATER POND 2 OUTLET

Worksheet

Worksheet for Circular Channel

Project Description	
Worksheet	Pond 2 outlet
Flow Element	Circular Chann
Method	Manning's Forr
Solve For	Channel Depth

Input Data	
Mannings Coeffic	0.013
Channel Slope	066700 ft/ft
Diameter	15.0 in
Discharge	1.88 cfs

Results	
Depth	0.28 ft
Flow Area	0.2 ft <sup>2</sup>
Wetted Perime	1.24 ft
Top Width	0.00 ft
Critical Depth	0.55 ft
Percent Full	22.7 %
Critical Slope	0.005436 ft/ft
Velocity	9.00 ft/s
Velocity Head	1.26 ft
Specific Energy	1.54 ft
Froude Numbe	3.55
Maximum Disc	17.95 cfs
Discharge Full	16.68 cfs
Slope Full	0.000847 ft/ft
Flow Type	supercritical

← USE INT. RIPRAP PAD

STORMWATER POND 2  
OUTLET

**OUTLET PROTECTION - OUTLET VELOCITY  $\leq$  14 feet/sec**

DISCHARGE (cfs)	OUTLET PIPE DIAMETER OR SPAN (in)									
	12	15	18	24	30	36	42	48	54	60
0-5	<b>10</b>	<b>10</b>		<b>USE</b>						
6	12	11								
7		13	<b>12</b>							
8		14	13	<b>12</b>			<b>MINIMUM</b>			
9			14	13						
10			15	13						
11			16	14					<b>LENGTH</b>	
12				14						
14				16	<b>14</b>					
16				17	15	<b>14</b>				<b>OUTLINED</b>
18				18	16	15				
20					17	15	<b>14</b>			
22		<b>USE</b>			18	16	15			
24						17	15	<b>14</b>		
26						17	16	15		
28						18	16	15		
30						19	17	16		
35						20	18	17	<b>16</b>	
40				<b>PREFORMED</b>			20	18	17	<b>16</b>
45							21	19	18	16
50							22	20	18	17
55								21	19	18
60								22	20	19
65								24	21	20
70						<b>SCOUR</b>		25	22	20
75								26	23	21
80									24	22
90									26	24
100									28	25
110										27
125							<b>HOLE</b>			29
130										30

**Table 8-6.1 - Length -  $L_a$  (feet)  
Type A Riprap Apron**

- Notes: 1. Bold face outlined boxes indicate minimum  $L_a$  to be used for a given pipe diameter or span.  
2. Rounding and interpolating are acceptable.

$$W = 3(S_p) + 0.4(L_a) = 7.75', \text{ use } 8'$$

8'W x 10'L INT. RIPRAP PAD

**TEMPORARY SEDIMENT TRAP SIZING  
PER 2002 CT DEP E&S MANUAL  
9-23-11**

**TST 1 (MAIN ACCESS DRIVE - STATION 20+00 – TURBINE 3 LOCATION):**

Initial Storage Volume = 134 cubic yards per acre of drainage area

$V = 134 \text{ cubic yards} \times 0.85 \text{ acres} = 114 \text{ cubic yards}$

Half of Storage Volume will be wet and half dry =  $114 / 2 = 57 \text{ cubic yards} = 1,539 \text{ cubic feet}$

$V_{\text{wet}} = 0.85 \times A_{\text{wet}} \times D_{\text{wet}}$

If trap is 2.0' deep:  $1,539 \text{ cubic feet} = 0.85 \times A_{\text{wet}} \times 2.0'$

$A_{\text{wet}} \text{ required} = 905 \text{ square feet}$

Use dimension of 20' x 46' for Wet Surface Area =  $A_{\text{wet}} = 920 \text{ square feet}$

$V_{\text{dry}} = (A_{\text{wet}} + A_{\text{dry}}) / 2 \times D_{\text{dry}}$

If dry area is 1.5' high,  $A_{\text{dry}} = 26' \times 52' = 1,352 \text{ sf}$

$V_{\text{dry}} = (920 + 1,352) / 2 \times 1.5' = 1,704 \text{ cubic feet}$

Use dimension of 26' x 52' for Dry Surface Area

**TST 2 (TURBINE 2 LOCATION):**

Initial Storage Volume = 134 cubic yards per acre of drainage area

$V = 134 \text{ cubic yards} \times 0.89 \text{ acres} = 119 \text{ cubic yards}$

Half of Storage Volume will be wet and half dry =  $119 / 2 = 59.5 \text{ cubic yards} = 1,606 \text{ cubic feet}$

$V_{\text{wet}} = 0.85 \times A_{\text{wet}} \times D_{\text{wet}}$

If trap is 2.0' deep:  $1,606 \text{ cubic feet} = 0.85 \times A_{\text{wet}} \times 2.0'$

$A_{\text{wet}} \text{ required} = 945 \text{ square feet}$

Use dimension of 20' x 50' for Wet Surface Area =  $A_{\text{wet}} = 1,000 \text{ square feet}$

$V_{\text{dry}} = (A_{\text{wet}} + A_{\text{dry}}) / 2 \times D_{\text{dry}}$

If dry area is 1.5' high,  $A_{\text{dry}} = 26' \times 56' = 1,456 \text{ sf}$

$V_{\text{dry}} = (1,000 + 1,456) / 2 \times 1.5' = 1,842 \text{ cubic feet}$

Use dimension of 26' x 56' for Dry Surface Area

**TST 3 (MAIN ACCESS DRIVE - STATION 10+00 – STORMWATER POND 1 LOCATION):**

Initial Storage Volume = 134 cubic yards per acre of drainage area

$V = 134 \text{ cubic yards} \times 4.85 \text{ acres} = 650 \text{ cubic yards}$

Half of Storage Volume will be wet and half dry =  $650/2 = 325 \text{ cubic yards} = 8,775 \text{ cubic feet}$

$V_{\text{wet}} = 0.85 \times A_{\text{wet}} \times D_{\text{wet}}$

If trap is 3.0' deep:  $8,775 \text{ cubic feet} = 0.85 \times A_{\text{wet}} \times 3.0'$

$A_{\text{wet}} \text{ required} = 3,441 \text{ square feet}$

Use dimension of 38' x 98' for Wet Surface Area =  $A_{\text{wet}} = 3,724 \text{ square feet}$

$V_{\text{dry}} = (A_{\text{wet}} + A_{\text{dry}})/2 \times D_{\text{dry}}$

If dry area is 3.0' high,  $A_{\text{dry}} = 56' \times 116' = 6,496 \text{ sf}$

$V_{\text{dry}} = (3,724 + 6,496)/2 \times 3.0' = 15,330 \text{ cubic feet}$

Use dimension of 56' x 116' for Dry Surface Area

\*Note – TST3 is significantly oversized because area will be rough graded to be used as Stormwater Pond post-construction.

**TST 4 (MAIN ACCESS DRIVE - STATION 6+00 – STORMWATER POND 2 LOCATION):**

Initial Storage Volume = 134 cubic yards per acre of drainage area

$V = 134 \text{ cubic yards} \times 1.62 \text{ acres} = 217 \text{ cubic yards}$

Half of Storage Volume will be wet and half dry =  $217/2 = 108.5 \text{ cubic yards} = 2,930 \text{ cubic feet}$

$V_{\text{wet}} = 0.85 \times A_{\text{wet}} \times D_{\text{wet}}$

If trap is 3.0' deep:  $2,930 \text{ cubic feet} = 0.85 \times A_{\text{wet}} \times 3.0'$

$A_{\text{wet}} \text{ required} = 1,149 \text{ square feet}$

Use dimension of 24' x 86' for Wet Surface Area =  $A_{\text{wet}} = 2,064 \text{ square feet}$

$V_{\text{dry}} = (A_{\text{wet}} + A_{\text{dry}})/2 \times D_{\text{dry}}$

If dry area is 2.5' high,  $A_{\text{dry}} = 39' \times 101' = 3,939 \text{ sf}$

$V_{\text{dry}} = (2,064 + 3,939)/2 \times 2.5' = 7,504 \text{ cubic feet}$

Use dimension of 39' x 101' for Dry Surface Area

\*Note – TST4 is significantly oversized because area will be rough graded to be used as Stormwater Pond post-construction.